- 1 Phase development and mechanical response of
- 2 low-level cement replacements with wood ash and
- 3 washed wood ash
- 4 Nina M. Sigvardsen^{a,*}, Mette R. Geiker^b, Lisbeth M. Ottosen^a
- 5 a Department of Civil Engineering, Technical University of Denmark, Kgs. Lyngby, Denmark
- 6 b Department of Structural Engineering, Norwegian University of Science and Technology, Trondheim, Norway

8 Abstract

7

19

20

- 9 A significant increase is seen worldwide in the amounts of wood ashes (WAs) produced by the
- 10 heating and power industry. This study investigates the potential utilisation of two different
- 11 WAs as low-level cement replacements, in an untreated and a washed version. Phase
- development and mechanical response were investigated. Results indicated ettringite formation
- mainly to contribute to the strength development until excessive formation. Indications of an
- optimum SO₃/C₃A ratio for mixtures with a low-level cement replacement with WA was
- between 0.4 and 0.5. Washed WA originating from grate combustion appeared to be most
- promising of the tested WAs in cement-based materials.
- 17 Keywords: Wood ash, low-level cement replacement, phase development, mechanical
- 18 response, cement-based material.

1. Introduction

- 21 Cement production accounts for 8-9% of the total anthropogenic CO₂ emissions [1], and the
- percentage is increasing due to a growing need for cement in developing countries [2]. One

E-mail address: nimasi@byg.dtu.dk (N.M. Sigvardsen).

Abbreviations (not standard): WA1: untreated wood ash no. 1, WA2: untreated wood ash no. 2, WA1-W: wood ash no. 1 exposed to a preliminary washing treatment, WA2-W: wood ash no. 2 exposed to a preliminary washing treatment.

^{*} Corresponding author.

way to decrease the associated CO₂ emissions from cement production is the use of blended cement where a part of the cement is substituted with other materials. The production of ash deriving from biomass, organic materials such as wood, is increasing due to a withdrawal of the traditional coal-fired power plants in Europe [3], substituting coal with biomass in the heating and power production [4]. Currently, approximately 10 million tonnes of biomass ash are produced annually worldwide alone from the electricity production, thus for heat-only production, additional quantities of biomass ash can be added to the quantities [4]. This leads to a series of potentially new types of materials which can be used as a partial cement substitution in blended cement. Such a potential new material could be wood ash (WA), originating from combustion of wood and wood products [5].

23

24

25

26

27

28

29

30

31

32

33

34

35

36

37

38

39

40

41

42

43

44

45

46

47

48

49

50

51

52

53

54

55

The performance of wood ash in mortar and concrete, studied in the literature [5–11], vary significantly depending on the physicochemical characteristics of the WA utilised, again depending on, e.g. the type of wood fuel and combustion process used for the energy production [12,13]. This is clearly seen, e.g. from compressive strength measurements of mortar samples with a partial cement replacement of cement with different WAs; Udoeyo et al. [11], Berra et al. [6], and Elinwa and Mahmood [10] all saw a decrease in the compressive strength with increasing replacement rates, while Rajamma et al. [14] and Ramos et al. [7] observed a maintained compressive strength for samples with 10 and 20% cement replacements compared to a reference sample. Further, the properties of a WA can be improved by different types of pre-treatments; Berra et al. [6] included a washing step, Doudart de la Grée et al. [15] included both a sieving and washing step, and Rosales et al. [16] included grinding, heating and removal of remnants of charred wood, all pre-treatments improving the compressive strength measurements of cement-based materials with WA as a partial cement replacement. Washing of the WA reduces the content of soluble salts (such as chlorides and sulfates) [15]. These promising results encourage further studies, including pre-treatments and following both the strength and the phase development over time, as the latter has not been included in any of the previous studies, in order to increase the knowledge on how pre-treatment of WA influences the two. Based on previous studies, only low-level replacements (10wt%) of cement with WA, have seen to be feasible based on the compressive strength. This study continues these investigations on low-level replacements in order to determine the effects on the compressive strength and phase development at low-level cement replacements with WA.

The objectives of this study are: (1) to determine the impact of sieving and washing treatment of the WAs on the physicochemical characteristics; (2) to investigate the impact of a low-level

cement replacement with WA on the mechanical response and phase development as a function of time; and (3) to determine the influence of sieving and washing treatments before utilisation of WA as a low-level cement replacement according to the strength and phase development.

In the presented study, two types of WA originating from different types of combustion but using the same type of biofuel were investigated. The two investigated WAs originate from combustion of wood chips. One originates from grate combustion, and the other from circulating fluidised bed combustion. Wood chips for the energy production are one of the most commonly used biomasses, and the use has increased in consumption over the last 20 years (841% increase in the consumption of wood chips for the energy production since 2000 [17]). According to a recently published report by representatives from Austria, Canada, Denmark, Germany, Italy and the Netherlands [4] grate combustion is predominately used for combustion of biomass, but circulating fluidised bed combustion is gaining ground in many of the countries. These types of plants, grate or circulating fluidised bed combustion, have the highest efficiency when solely biomass is used [18], and are the most common type of plants regarding combustion of biomass. This means that the two chosen WAs represent common types of WAs.

2. Materials and methods

2.1. Materials

Portland, Denmark (CEM I), inert quartz (Qz) [19], and two types of wood fly ash, sampled at Skærbækværket Biomass Power Plant, Denmark (WA1) and Värtaverket Combined Heat and Power Plant, Sweden (WA2). WA1 is a residue from grate combustion (600-1,000°C). WA2 is a residue from circulating fluidised bed combustion (760-930°C). The fuel used at both plants was a similar type of wood chips originating from whole trees, primary pine trees, including bark and needles [20]. After sampling, the WAs were stored in closed plastic buckets protected from moist, heat and light sources. The buckets were rotated by hand in order to increase the uniformity of the WA before smaller portions of WA for further treatment were sampled. The WA samples were divided into two portions. The WAs were dried at 50°C and sieved. The sieving removed the particles ≥ 250µm, which consisted only of larger remnants from charred wood. The larger remnants of charred wood accounted for approximately 25% of WA1 and < 2% of WA2 when received from the power plants. Smaller particles of charred wood, which

- can be found in the fraction $\leq 250 \mu m$ [15], are not removed. One portion of each WA was
- subjected to drying at 50°C and sieving to a particle size of $\leq 250 \mu m$ (termed WA1 and WA2).
- 89 The other portion was washed after drying and sieving (termed WA1-W and WA2-W). The
- 90 washing procedure was as follows: 100 g ash was mixed with distilled water to a liquid-to-
- 91 solid ratio (L/S) of 5 and shaken manually for 1 min [21]. After settling (app. 5 min.), the water
- was decanted. This procedure was repeated three times, the suspension was filtered (retention
- 93 12-15µm), and the ash was subsequently dried at 50°C.
- 94 *2.2 Methods*
- 95 2.2.1 WA characterisation
- 96 Selected characteristics of the WAs, CEM I and Qz, are given in Tab. 2. Loss on ignition (LoI)
- 97 was measured at 950°C, according to EN 196-2 [22]. The element content was determined by
- 98 X-ray fluorescence (XRF) spectrometry measured by the use of a SPECTRO Gmbh X-LAB
- 99 2000 with a Pd-tube on samples ground to a particle size of less than 200µm. The software
- used was TQ-3945r, and the equivalent content of oxides was calculated based on the element
- 101 content. Cl⁻ and SO₄⁻² were measured with Ion Chromatography (IC) on a 1:2.5 solid-to-liquid
- ratio suspension in distilled water after 1 hour of agitation. The free calcium oxide contents in
- the WAs were measured according to EN 451-1 [23].
- The particle size distribution was found by laser diffraction by the use of Mastersizer 2000.
- The pH was measured for all materials in 1:2.5 solid-to-liquid ratio suspensions in distilled
- water with a pH electrode after 1-hour agitation. The electric conductivity was determined on
- the same suspension with an electrical conductivity meter. The particle density was measured
- by the use of a helium gas pycnometer Micrometics AccuPyc 1340 in accordance with EN 196-
- 109 6 [24].
- The crystalline phases in the materials were identified by XRD. The WA was loaded into the
- sample holder using backloading. A PanAnalytical X-ray diffractometer, sat at the PW3064
- Spinner stage for standard powder samples have been used for the measurements. Cu-Kα
- 113 radiation with a wavelength of 1.54 Å was used as the x-ray source. The samples were
- measured between 4 °2 θ and 100 5 °2 θ with a step size of 0.002 °2 θ and a sampling time per
- step of 24.8 s. The XRD plots were qualitatively evaluated using X'Pert HighScore Plus
- 116 software, with data from the International Centre for Diffraction Data (ICDD). A
- semiquantitative analysis has been performed in X'Pert HighScore Plus software, determining

- approximate amounts (in %) of the crystalline phases. The detection limited of the XRD
- analysis is 2 wt%. Grain morphology was analysed by the use of Scanning Electron Microscope
- 120 (SEM)-images performed on a FEI Quanta 200. The pictures were magnified X1500, and the
- accelerating voltage of the beam was 20 keV with a large field detector under low vacuum
- 122 conditions.
- 123 The thermogravimetric analysis (TGA) was performed on both WAs and WA-Ws.
- 124 Approximate 40 mg of the sample was poured into 85 μl aluminium oxide crucibles (diameter
- 6.8mm), and the weight loss was measured from room temperature to 900 °C in a NETZSCH
- 126 STA 449 F3 Jupiter. First, the temperature was raised to 40 °C and held for 10 min.; then the
- temperature was increased to 900 °C with a heating rate of 10 °C/min. During the
- measurements, the cell was purged with 50 ml/min of nitrogen gas. The data were processed
- using the analysis software Proteus Analyzer. The unburned carbon content was determined
- from LoI and TGA measurements.
- 131 2.2.2. Paste preparation and double solvent exchange
- Six paste mixes were prepared, one reference and with 10% replacements of cement either with
- one of the four WAs or Qz with a constant w/b = 0.5. The pastes were mixed using a high
- shear mixer (Whip Mix Power Mixer Model B). The mixing procedure was adapted from EN
- 135 196-1 [25]: mixing for 90 s, resting for 90 s, and mixing for 60 s. The pastes were cast in 5 ml
- polythene tubes (diameter 12 mm) and stored for one day in a temperature-controlled room
- 137 (20°C, <80% RH). The pastes were demoulded and transferred into larger 25 ml vials (diameter
- 138 25 mm), subsequently filled with lime water (3 g calcium hydroxide/L distilled water [26]) and
- stored in a temperature-controlled room (20°C). The pastes were tested after 1, 3, 7, 14, 28, 90,
- 140 182 and 365 days of hydration.
- Double solvent exchange was used for hydration stoppage [27]. Four 2 mm thick slices
- (diameter 12 mm) were cut from the middle of the cured paste samples. The slices were crushed
- in a porcelain mortar until the whole paste sample could pass through a 1 mm sieve.
- 144 Approximately 3g of the crushed sample was immersed in 50 mL isopropanol. The suspension
- was shaken for 30 sec, left to rest for 5 min, and subsequently decanted. This isopropanol
- treatment was performed twice, and then the suspension was vacuum-filtered. The crushed
- sample was then immersed in 10 mL diethyl ether, shaken for 30 sec, left to rest for 5 min., and
- vacuum-filtered [27]. The crushed cement paste (< 1mm) sample was shortly subjected to an

149 oven drying process (8 min. at 40°C) in order to remove the easily volatile diethyl ether. This 150 method does not alter the hydrate products significantly [27]. For the TGA and XRD analysis, 151 the paste samples were further crushed in a porcelain mortar, right before test execution, until the whole paste sample could pass through a 63µm sieve [27]. 152

153 2.2.3. Thermogravimetric analysis (TGA)

154

155

156

157

158

159

160

161

162

163

164

165

166

167

168

169

170

171

172

173

Thermogravimetric analysis (TGA) was performed on pastes subsequently to the double solvent exchange treatment, drying and crushing. The TGA was performed as described in section 2.2.1. From the TGA measurements, the loss of bound water and decomposition of ettringite and calcite were determined by the use of a horizontal step and decomposition of portlandite was determined by the use of a tangential step. The step for bound water was 50°C and 550°C [27], for ettringite 50°C and 120°C [28], for portlandite 400°C and 550°C [27], and for calcite 550-800°C [27], respectively. It should be noted that the temperature interval for ettringite also includes carbonate AFm phases and C-S-H [28]. However, the included mass loss due to carbonate AFm phases was expected to be very small as the main peak of the carbonate AFm phases was determined to be above 120°C (approximately 160°C) as displayed in Fig. 5. The mass loss due to decomposition of C-S-H cannot be separated from the mass loss corresponding to ettringite as the TGA peaks overlap. This might lead to an overestimation of the amount of ettringite formed. However, this is acceptable as the WAs are not expected to contribute significantly to the formation of C-S-H compared to the formation of ettringite [29] due to a low content of silica in the WA, and the increase due to C-S-H can therefore mainly be attributed to the cement hydration. The equations for the quantification of the bound water (H), portlandite (CH), ettringite ($C_6As_3H_{32}$) and calcite (\overline{CC}) relative to the dry mass or clinker content (c.f. [27]) are given in Eqs. (1)-(12).

$$H_{measured} = WL_{50-550} \tag{1}$$

$$H_{measured} = WL_{50-550}$$

$$H_{anhydrous} = \frac{H_{measured}}{weight \ at \ 550^{\circ}C}$$
(1)

$$H_{Clinker} = \frac{H_{measured}}{weight \ at \ 550^{\circ}C} \times \frac{100}{\% clinker}$$
 (3)

$$CH_{measured} = WL_{400-550} \cdot (74/18)$$
 (4)

$$CH_{measured} = WL_{400-550} \cdot (74/18)$$

$$CH_{anhydrous} = \frac{CH_{measured}}{weight \ at \ 550^{\circ}C}$$
(5)

$$CH_{Clinker} = \frac{CH_{measured}}{weight \ at \ 550^{\circ}C} \times \frac{100}{\% clinker}$$
 (6)

$$C_6 As_3 H_{32_{measured}} = WL_{50-120} \cdot (1255/(32 \cdot 18)) \tag{7}$$

$$C_{6}As_{3}H_{32_{measured}} = WL_{50-120} \cdot (1255/(32 \cdot 18))$$

$$C_{6}As_{3}H_{32_{anhydrous}} = \frac{C_{6}As_{3}H_{32_{measured}}}{weight \ at \ 550^{\circ}C}$$
(8)

$$C_6 As_3 H_{32}_{clinker} = \frac{C_6 As_3 H_{32}_{measured}}{weight at 550^{\circ}C} \times \frac{100}{\% clinker}$$
(9)

$$C\overline{C}_{measured} = WL_{550-800} \cdot (100/44) \tag{10}$$

$$C\overline{C}_{measured} = WL_{550-800} \cdot (100/44)$$

$$C\overline{C}_{anhydrous} = \frac{C\overline{C}_{measured}}{weight \ at \ 550^{\circ}C}$$
(10)

$$C\overline{C}_{Clinker} = \frac{\text{CaCO}_{3_{measured}}}{\text{weight at } 550^{\circ}\text{C}} \times \frac{100}{\text{\%clinker}}$$
(12)

- 176 The standard deviations of these quantifications were based on three independent
- 177 measurements and quantifications of the bound water (H), portlandite (CH), ettringite
- $(C_6As_3H_{32})$ and calcite (\overline{CC}) of a control sample at 28 days of hydration. 178
- 179 2.2.4. X-ray diffraction (XRD)
- 180 X-ray diffraction (XRD) analysis was performed on the same pastes used for the TGA
- subsequently to the double solvent exchange treatment, drying and crushing. The XRD analysis 181
- 182 was performed, as described in section 2.2.1. Semiquantitative analysis was performed in
- 183 X'Pert HighScore Plus software, determining approximate amounts (in %) of the crystalline
- 184 phases.

- 185 2.2.5. Preparation of mortar samples and compressive strength testing
- Mortar mixes were prepared according to the mixing procedure prescribed in EN 196-1 [25] 186
- 187 with a constant w/b = 0.5. One control mix without WA (REF), five mixes with 10%, and five
- mixes with 20% replacements of cement (in accordance with ASTM C311/C311M-13 [30]), 188
- 189 either with one of the four WAs or Qz. WA or Qz was added to the unhydrated cement before
- 190 mixing. Super plasticiser (Dynamon XTend from Mapei) was added to the mixing water and
- 191 in order to achieve standard consistency, thus keeping a constant w/b ratio. The consistency
- 192 was determined by flow table according to EN 1015-3 [31] for mixes containing WA or Qz,
- 193 see Tab. 1. Mortar prisms (40 x 40 x 160 mm) were cast in accordance with EN 196-1 [25].
- 194 Three mortar prisms were prepared for each of the mixes for each of the hydration ages. After
- 195 1 day in a temperature-controlled room (20°C, <80% RH) the three prisms from each mix were
- 196 demoulded and stored in separate boxes submerged in lime water (3 g calcium hydroxide/L

distilled water [26]) in a temperature-controlled room (20°C). The mortar specimens with 10% replacements were tested after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration and the mortar specimens with 20% replacements were tested after 7 and 28 days of hydration [32].

After hydration, the compressive strength of the mortar specimens was determined according to EN 196-1 [25]. The mortars were split in two with an electro-mechanic test machine (Instron 6022) by increasing the load by 0.05 kN/s [25], and the compressive strength was measured for all the subsequent six pieces of mortar with a servohydraulic test machine (Toni Technik 300Ton) by increasing the load by 2.4 kN/s [25]. The air content of mortars was determined in accordance with Osbæck [33] by the use of the weight at demoulding and the theoretical air void free mortar. The compressive strength was then normalised to an air content of 2 vol% by the use of Bolomeys equation [34]. SAI was calculated according to ASTM C311/C311M-13 [30].

3. Results

- 3.1 Characteristics of WAs and Qz
- DTG curves for all materials are displayed in Fig. 1. Some prehydration of CaO resulting in portlandite formation were seen for the washes ashes as a result of the washing treatment. The chemical composition of the materials is presented in Tab. 2 and the physical properties are presented in Tab. 3. A total below detection limit of Cl^- and SO_4^{2-} and a reduction in the content of alkali metals K_2O and Na_2O are results from the washing of WAs. Tab. 4 displays the crystalline phases for the materials and hydration of CaO to Ca(OH)₂ is seen for washed WAs. The grain morphology is shown by SEM images (Fig. 3). This reveals large, angular particles for both WAs. A layer of soluble compounds covers the untreated WAs, Fig. 3 (a) and (c), seen as minor particles covering larger particles. When the WAs were washed, Fig. 3. (b) and (d), this layer was removed. More spherical particles were found in WA1-W compared to WA2-W. The washing did not improve LoI of the WAs, which are high ($\geq 15\%$) for all WAs. The unburned carbon content was $\leq 1\%$ for both WA1 and WA1-W and 6 and 7% for WA2 and

225 3.2. Strength activity index

WA2-W respectively.

- For pozzolanic activity, the ASTM C618 [32] requires an SAI above 0.75 compared to the
- reference after 7 and 28 days of hydration for a mortar sample containing 20% possible
- pozzolan and 80% CEM I as binder. The SAI are presented in Tab. 5 and only WA1 mortar
- does not comply with the limit. The washing treatment leads to an increase in the SAI for both
- 230 types of WAs. Qz also complies with the limit, even though it being completely inert.
- 231 3.3. Compressive strength development
- 232 At 1 day of hydration, the compressive strength of the WA1 mortar is comparable to REF, see
- Fig. 4 and Tab. 6. The rest of the WA mortars obtains a slightly lower compressive strength,
- and a tendency is seen, where the washed WA mortars are more comparable with the Qz mortar.
- 235 At 3 days the compressive strength for all WA mortars is comparable to REF and higher than
- 236 the Qz mortar. From 7 days of hydration, the compressive strength of REF exceeds all WA and
- Qz mortars. At $90 \le$ days of hydration, all WA mortars have a slightly higher compressive
- 238 strength compared to the Qz mortar. The WA1-W mortar exceeds the compressive strength of
- all other WA mortars, followed by, in decreasing order, the WA2, WA1 and WA2-W mortar.
- 240 3.4. Phase development
- The phase development was analysed by XRD and TGA. Tab. 7 is an overview of the detected
- 242 phases in the pastes determined by XRD. Depletion of alite was seen after 3 days of hydration
- for all pastes. Depletion of belite and ferrite are seen after 14 days of hydration for WA1-W
- and Qz paste, and for the remaining pastes after 28 days of hydration. Portlandite and ettringite
- 245 were detected in all pastes from 1 days of hydaration and monocarbonate was detected in all
- pastes from $14 \le$ days of hydration. Quartz is detected in WA, and Qz pastes, at all hydration
- ages. Calcite is detected at 1 day of hydration for all WA pastes. However, dissolution of the
- 248 calcite is seen for WA2 and WA2-W pastes since no calcite is detected again before 90 days
- of hydration.
- 250 Differential thermogravimetric (DTG) curves, determined with TGA for all pastes at 28 days
- of hydration, are presented in Fig. 5. The DTG curves show peaks of weight changes related to
- 252 the decomposition of ettringite (Et) and C-S-H, monocarbonate (Mc) and portlandite (CH).
- 253 Above 550°C, calcite decomposes by emitting CO₂ [27], substantiating the change from
- 254 hydration to carbonation to be at 550° C as used in the formulas (1) (12).

- Quantification of bound water, portlandite, ettringite/C-S-H and calcite in all pastes are
- presented in Fig. 6, 7, 8 and 9, (a) calculated as wt%/anhydrous binder weight and (b) calculated
- as wt%/clinker content. The standard deviation was 0.67% wt for the quantification of bound
- 258 water, 0.56% wt for the quantification of portlandite, 0.27% wt for the quantification of
- ettringite and 0.33% wt for the quantification of calcite. This is illustrated as error bars in the
- 260 figures 6-9.
- 261 *3.4.1. Bound water*
- 262 At 1-3 days of hydration, a slightly larger amount of water was bound in the WA pastes
- 263 compared to REF, see Fig. 6 (a)). After 7 days of hydration, the amount of bound water in REF
- and WA pastes were comparable. At 14 and 28 days of hydration, the amount of water bound
- in REF slightly exceeds the amount bound in the WA pastes. At 365 days the amount of bound
- water in REF and WA pastes are in the same range, but with a slightly higher amount of bound
- water in WA2 and WA2-W pastes compared to WA1 and WA1-W pastes. Qz had the lowest
- 268 content of bound water of all pastes.
- 269 *3.4.2. Portlandite*
- 270 At 1-3 days of hydration, the portlandite content in WA and Qz pastes is similar or slightly
- 271 higher than REF (Fig. 7 (a)). At 7 days, the portlandite content for REF paste increases
- compared to WA and Qz pastes, which remains comparable for the rest of the hydration period.
- 273 *3.4.3. Ettringite*
- 274 Ettringite has been determined for both REF and WA pastes at all ages (Tab. 7). A comparable
- ettringite content (Fig. 8 (a)) is seen for WA pastes from 1 to 7 days of hydration. From $14 \le$
- 276 days of hydration, the ettringite content in the WA pastes exceeds REF. This is especially
- notable for WA2 paste from 90 days of hydration and, at later ages ($182 \le \text{days}$ of hydration),
- the same is seen for WA1 paste. Qz had the lowest content of ettringite at all ages.
- 279 *3.4.4. Calcite*
- A clear difference is seen in the calcite content between WA1 and WA1-W pastes, and WA2
- and WA2-W pastes at all ages (Fig. 9 (a)), with WA1 and WA1-W pastes having the highest
- content of calcite. The calcite content decreased from 1-90 days of hydration for all pastes
- followed by an increase after 90 days.

284 3.4.5. Dilution effect

In the pastes, the most reactive part (the cement) is replaced with a less or non-reactive material (WA or Qz). In order to take this dilution effect into account when analysing the results, the amount of bound water, portlandite, ettringite and calcite are calculated as wt%/clinker content, see Fig. 6, 7, 8 and 9 (b). The WA pastes have a higher amount of bound water than REF, and Qz pastes at all ages, see Fig. 6 (b). From 28 days of hydration, a slight increase in the portlandite content is seen for REF compared to WA and Qz pastes (Fig. 7 (b)). For the ettringite (Fig. 8 (b)) and calcite content (Fig. 9 (b)), Qz exceeds the contents calculated for REF.

4. Discussion

- In the following section, the objectives 1-3 outlined in section 1 will be discussed based on the results presented in section 3.
- 297 4.1. Impact of the washing on the physicochemical characteristics of the WAs
 - Washing of the WAs resulted in a slight increase of the particle size distribution (Fig. 2) by removing the soluble fraction consisting of compounds with smaller particle sizes. This is supported by the SEM images (Fig. 3). Fig. 3 (a) and (c) reveals a layer of smaller compounds covering the particles of the raw WAs. These compounds are removed by washing, see Fig. 3. (b) and (d), and are thus assumed soluble, probably salts with Cl⁻ and SO₄²⁻ and the alkali metals, K and Na such as KCl, K₂SO₄ (Tab. 4) and NaCl, Na₂SO₄, as these chemical elements are removed to below detection limit or significantly reduced (Tab. 2). No decrease of neither LoI nor the unburned carbon content is seen as a result of the washing. The sieving effectively removed unburned carbon from WA1 and WA1-W, however, smaller particles (≥ 250µm) of unburned carbon are found in WA2 and WA2-W, containing 6 and 7% unburned carbon, respectively. The unburned carbon can lead to an increase in the water requirement [12,35,36]. However, this was not seen for the WAs investigated in this study. The high LoI measured for all WAs are primarily due to the content of hydrate phases and especially carbonate phases and not a high content of unburned carbon. The content of free CaO is decreased by the washing, however, it could still present an issue regarding soundness of the blended cement.

Several other issues are possible when utilising these WAs as a partial cement replacement, e.g. P₂O₅ can cause delayed set, alkalis can cause alkali silica reactions and SO₃ can cause sulfate attack. In relation to the durability, washing improves the possible use of WAs as a partial cement replacement, but further treatments will possibly be required before utilisation in cement-based materials is possible. However, as these issues are not investigated in this study, this needs to be addressed further in a subsequent study.

The washing further caused a difference in the phase composition of the WAs. From the DTG curves (Fig. 1) of the unhydrated materials, C-S-H (as no ettringite was determined by XRD) and an increase in the calcite content can be found in the washed WAs. The increase in calcite is substantiated by the XRD (Tab. 4.) and is due to removal of other water-soluble compounds and carbonation. Portlandite has been determined for all WAs by the XRD (Tab. 4.), but the DTG curves (Fig. 1) reveals a difference in the impact of the washing on the Portlandite content between the two types of WAs. The content of portlandite increases after the washing of WA1, but decreases for WA2. For both washed WAs, a chemical prehydration process between CaO and the content of water forming portlandite following reaction (13) is expected [29], as no CaO are found in the washed WAs (Tab. 4.) and a significant reduction is seen in the content of free CaO (Tab. 2) are seen. Subsequently, carbonation occurs in WA2-W, decreasing the content of portlandite and leading to calcite formation, following reaction (14) [37–39].

$$CaO + H_2O \rightarrow Ca(OH)_2 \tag{13}$$

$$Ca(OH)_2 + CO_2 \to CaCO_3 + H_2O \tag{14}$$

- The initial content of portlandite and calcite in the untreated ashes are a consequence of water and CO₂ in the air during ash storage at the power plants, following reactions (13) and (14).
- Arcanite (K₂SO₄) was determined in WA1 by XRD WA1, see Tab. 4. Formation of arcanite in the ash is due to a higher release of alkali metals by volatilisation facilitated by the high temperatures in grate combustion, compared to a fluidised bed system [40]. Arcanite is watersoluble and is removed during the washing, and thus it is not present in WA1-W (Tab. 4).
 - Sylvite (KCl) and anhydrite (Ca(SO₄)) were identified in WA2 by XRD, see Tab. 4. Thus, in WA2 potassium is primarily bound with chloride rather than sulfate, and sulfate is bound primarily to calcium. Anhydratie is a result of a sulphation process at the power plant were limestone are added to capture SO₂, an important process in a fluidised bed system,

precipitating anhydrite [40]. Neither sylvite nor anhydrite is present in WA2-W (Tab. 4)

because these compounds are water-soluble, and the lime present in WA2 is hydrated to

- 345 portlandite following reaction (13).
- 346 The initial presence of calcite has a beneficial impact on the properties of a cementitious
- material due to the potential formation of hemi- and monocarbonate. These AFm phases are
- more stable than monosulfate in the presence of carbonates [41], and as a consequence,
- ettringite does not transform into monosulfate, but is stabilised. Stabilisation of ettringite leads
- 350 to an increase in compressive strength, as ettringite has a lager content of hydrate water and is
- more voluminous, leading to a reduction in the porosity [29,42–44]. Reactions (13) and (14)
- can occur during hydration of cement paste with partial cement replacement with WA due to
- 353 hydration of CaO and subsequently carbonation [39], as CaO is present in both WA1 and WA2
- and Ca(OH)₂ is present in all WAs (Tab. 4.).

355

356

- 4.2. Impact of the partial cement replacement of cement with WA on SAI, mechanical response
- 357 and phase development
- 358 *4.2.1. Strength activity index*
- From the results from the SAI test (Tab. 5), WA1-W, WA2 and WA2-W are seen to possess
- pozzolanic properties, meeting the requirements set by ASTM C618 [32] at both 7 and 28 days
- of hydration. A decrease of 20% and 15% decrease in the strength compared to REF are seen
- for Qz at 7 and 28 days of hydration, respectively, thus Qz complies with the requirements set
- by ASTM C618 [32]. However, Qz is completely inert and does not contribute through either
- nucleation processes or particle packing, due to the large particle size of Qz comparted to CEM
- 365 I (Fig. 2) [45].
- 366 The SAI test an indirect method measuring a physical property indicating the extent of
- pozzolanic activity of a material [46]. According to the literature [47–49], there are several
- 368 drawbacks of this method. An unreactive material can be misread as having pozzolanic
- properties, e.g. due to nucleation processes and particle packing effects [50,51] and other
- 370 hydrate phases contributing to the compressive strength cannot be distinguished from the
- 371 hydrate phases resulting from pozzolanic activity. Further ASTM C618 [32] requires
- 372 measurements only at 7 and 28 days of hydration, which for some types of pozzolans is

inadequate, e.g. coal fly ash shows very little pozzolanic reactivity 28 days of hydration [38]. Amorphous silica and aluminosilicate leads to pozzolanic activity [52]. Due to the WAs consisting of large and fibrous particles (Fig. 3) and not the typical glassy aluminosilicate spherical particles found in commercial coal fly ash [53], the amount of amorphous glass are considered to be limited in the WAs. Further, the content of silica in WA1 and WA1-W are low (< 13%), see Tab. 2. The silica content in WA2 and WA-2-W is higher (23.8 and 26.5%, respectively), but this can be attributed to inert sand particles, which make the suspension bed and are carried with the flue gas during combustion with the circulating fluidised bed technology ending up in the fly ash fraction [40]. Thus a very limited pozzolanic reaction with silica [43] can be expected and the increase in strength can mainly be due to the WAs contributing through hydration of other hydrate phases, like ettringite, contributing to an increase in compressive strength [42]. No contribution through heterogeneous nucleation processes from WAs are expected either due to their particle size distribution being equivalent or larger than CEM I, see Fig. 2 [45].

387

388

373

374

375

376

377

378

379

380

381

382

383

384

385

386

4.2.2. Mechanical response

- The development of compressive strength, when WAs are used as a partial cement replacement, is seen in Fig. 4. At 1 day of hydration WA1 and WA2 mortars have compressive strength comparable to the compressive strength of REF, and higher compressive strength compared to the mortars with WA1-W, WA2-W and Qz. The compressive strength of the mortar with WA1 is comparable to REF at both 1 and 3 days of hydration. All mortars containing WA, has a
- 394 slightly higher compressive strength than the mortar with Qz at 1-7 days of hydration.
- No contribution through heterogeneous nucleation processes from the WAs are expected, as
- described in section 4.2.1, thus the increase in the compressive strength of WA mortars being
- 397 comparable to REF and not Qz at yearly age indicates an influence from chemical reactions
- 398 due to dissolution of easily soluble salts leading to strength improving hydration products
- 399 [29,42–44].
- 400 At late age, $90 \le$ days of hydration, all WA mortars have a higher compressive strength than
- 401 the Qz mortar. The low compressive strength of Qz is due to the inert nature of the Qz and a
- low density (2650kg/m³ compared to 3180kg/m³ for CEM I), resulting in a larger volume based

- 403 addition of Qz, leading to relatively less space available for the hydrate formation further
- 404 decreasing the strength.
- 405 At 365 days of hydration the average compressive strength for mortar with WA1 arrives at
- 406 86%, WA2 at 87%, WA1-W at 91%, WA2-W at 84% and Qz at 82% compared to the
- 407 compressive strength of REF. The lower compressive strength of WA mortars reflects the
- dilution effect, decreasing the amount of hydrated cement and the larger volume based addition
- of the WA compared to CEM I. However, the increase in compressive strength compared to
- 410 Qz indicates that the WA mortars facilitates the development of phases contributing to an
- 411 increase in the compressive strength.
- 412 *4.2.3. Phase development*
- 413 *4.2.3.1. Bound water*
- An increase in the amount bound water is seen for WA pastes at 1-3 days of hydration (Fig. 6
- 415 (a)) compared to REF. Such increase can be explained by the dissolution of easily soluble salts
- enhancing the initial growth of hydrated phases. From 14 to 90 days of hydration, the bound
- 417 water in REF exceeds all WA pastes, but at 365 days of hydration, all WA pastes have
- 418 approximately the same amount of bound water as the REF, indicating a continuous hydration
- of phases in the WA pastes compared to REF. WA2 and WA2-W pastes contain a slightly
- higher amount of bound water compared to WA1 and WA1-W pastes. This is explained by
- WA2 and WA2-W binding more water in phases such as ettringite (see Fig. 8).
- The amount of bound water for WA pastes exceeds Qz at all ages (Fig. 6 (a)). These
- observations correspond to the WAs contributing through a quick initial dissolution of the
- 424 particles at early age and development of phases contributing to the compressive strength at
- 425 later ages.
- 426 *4.2.3.2. Portlandite*
- Initially (1-7 days), the content of portlandite for all pastes with WAs exceeds the content of
- portlandite for pastes with Qz (Fig. 7 (a)). The increase in portlandite, compared to Qz, can be
- 429 explained by an additional formation of portlandite due to a reaction between CaO in the WAs
- 430 and H₂O (Eq. 13).

- After 28 days of hydration, the portlandite content in WA pastes is slightly exceeded by the
- portlandite content of Qz when the dilution effect is considered (Fig. 7 (b)). This is explained
- by consumption of portlandite in the WAs either as a result of, to a minor extent, the pozzolanic
- 434 reaction or formation of ettringite occurring at the expense of portlandite, aluminium and
- 435 sulfate [41].
- 436 *4.2.3.3. Ettringite/C-S-H*
- 437 As discussed previously (section 2.2.3.) the formation of C-S-H cannot be separated from the
- mass loss corresponding to ettringite as the TGA peaks overlap, thus ettringite discussed in this
- 439 section can include C-S-H. Some initial C-S-H is determined for washed WAs, however, based
- on the discussions presented in section 4.2.1. and 4.2.2. the WAs are not expected to contribute
- significantly to the formation of C-S-H compared to the formation of ettringite, substantiated
- by the literature [29]. Further, ettringite has been determined for both REF and WA pastes by
- 443 XRD at all ages (Tab. 7).
- A difference is seen at 1 day of hydration for the ettringite content between pastes with
- untreated and washed WAs (Fig. 8), corresponding to the initial C-S-H content and not
- ettringite found in WA1-W and WA2-W (Fig. 1.) A rapid increase in the ettringite content is
- seen for WA1 and WA2 from 1 to 3 days of hydration, corresponding to the dissolution of
- 448 SO_4^{2-} in the untreated ashes (Fig. 8.).
- All WA pastes have a higher content of ettringite than Qz at all ages and REF from 28 days of
- 450 hydration, see Fig. 8 (a). The increase mainly contributed to the ettringite formation in WA
- pastes is attributed to an increase in the SO₃/C₃A ratio [38] and not formation of C-S-H. The
- content of ettringite was highest in the paste with WA2, corresponding well to WA2 having
- 453 the largest initial content of SO₃ (Tab. 2). WA1 has the second-largest initial content of SO₃
- and the second-largest content of ettringite at $182 \le \text{days}$ of hydration. These tendencies are
- consistent with the increased amount of bound water (Fig. 6 (b)) in WA pastes. Due to the
- amount of hydrate water, ettringite is voluminous, which leads to a reduction in the porosity
- and consequently to an increase in compressive strength [29,42–44]. At $182 \le days$ of
- 458 hydration, the washed WA pastes have a lower ettringite content, compared to the untreated
- WAs, due to a lower SO₃ content (Tab. 2).
- There was no decrease in the ettringite content due to stabilisation as a result of the presence
- of calcite and formation of monocarbonate (determined by the TGA, Fig. 5, and XRD Tab. 7.).

462 *4.2.3.4. Calcite*

- A significant difference in the content of calcite at 1 day of hydration is seen between pastes
- with WA1 and WA1-W, WA2 and WA2-W, and Qz and REF, see Fig. 9 (a). This is due to a
- difference in the initial content of the materials: calcite contribution from WA and 5 %
- limestone filler in CEM I. WA1 and WA1-W pastes have approximately 2 wt% higher content
- of calcite than WA2 and WA2-W pastes (Fig. 9 (a)), attributed to a higher initial content of
- 468 calcite in WA1 and WA1-W (Fig. 1).
- A decrease in calcite content is seen for all pastes from 1-90 days of hydration, especially
- 470 profound for WA pastes, due to formation of mono- and hemicarbonate at the expense of calcite
- 471 (determined by the TGA, Fig. 5, and XRD Tab. 7.). An increase in the calcite content is seen
- from $90 \le$ days of hydration (Fig. 9 (a) and (b)), due to carbonation at later ages for WA pastes.

473

- 4.3. Impact of washing on phase and compressive strength development
- Several differences were seen between the two types of WAs, before and after washing when
- 476 utilised in as low cement-replacements in cement-based materials. More ettringite is found in
- pastes with untreated WA, compared to pastes containing washed WA, due to a larger SO₃
- 478 content in untreated WAs. The washing leads to a higher content of calcite due to removal of
- 479 soluble and carbonation occurring during washing. In general, the contribution to the
- 480 compressive strength from the WAs is mainly due to the formation of ettringite as a result of
- an increased SO₃/C₃A ratio and stabilisation by calcite forming monocarbonate [41]. However,
- excessive formation of ettringite can lead to micro-cracking and a decrease in strength if the
- stress exceeds the tensile strength of the binder [38]. This can explain the different results
- obtained for WA mortars, where WA1-W obtains a compressive strength above 90% the
- reference strength and the compressive strength of WA1, WA2 and WA2-W all arrive at
- 486 between 84-87%.
- 487 A sufficient amount of ettringite is precipitated in WA1-W in order to increase the strength,
- but low enough not to facilitate micro-cracks. The content of C₃A in the clinker can be
- calculated by the use of Bogue calculations based on the content of Al₂O₃ and Fe₂O₃ [41], and
- 490 the SO_3/C_3A ratios are calculated for the WAs: WA1 = 0.5, WA1-W = 0.4, WA2 = 0.5, and
- WA2-W = 0.5. This indicates the optimum SO₃/C₃A ratio of a mixture with a partial cement

replacement with WA to be: $0.4 \le$ optimum < 0.5. This provides an indication of a sufficient removal of the SO₃ from a WA by a washing treatment order to obtain a sufficient, but not excessive amount of ettringite contributing positively to the strength development. The washing removed 76% of the SO₃ from WA1 and only 39% of SO₃ from WA2, thus further treatment should be conducted for WA2-W indicating WA1-W to be more promising as a low-level partial cement replacement.

Again, it should be noted that washing improves the possible use of WAs as a partial cement replacement, but further treatments will possibly be required before utilisation in cement-based materials is possible, especially considering several possible durability issues.

5. Conclusion

Two wood ashes (WAs), originating from two different power plants using circulating fluidised bed combustion and grate combustion, respectively, were investigated in an untreated as well as washed version for the purpose of low cement replacements (10%) in cement-based materials. The time-dependent development of bound water, portlandite, ettringite(/C-S-H) and calcite were measured in cement pastes with 10% replacement with the WAs. In general, increased content of bound water, ettringite and calcite, and decreased content of portlandite were seen for all pastes with WA compared to the Portland cement reference.

If only the mechanical response is considered, the results indicate that the WAs contributes to a minor extent to the compressive strength through pozzolanic reactions and the main contribution to the compressive strength are determined to be the ettringite content. However, excessive formation of ettringite leads to micro-cracking and a decrease in strength. Thus, the optimal ettringite content, contributing to the strength and not to micro-cracking, are found based on the SO₃/C₃A ratio for mixtures with low-level cement replacement with WA between 0.4 and 0.5.

For the two WAs investigated in this study, washed WA originating from grate combustion appears to be more promising as a low-level cement replacement a cement-based material compared to both untreated and washed WA originating from circulating fluidised bed combustion based on the compressive strength development.

527

6. Acknowledgement

- 523 The reported work was supported by the Department of Civil Engineering at the Technical
- 524 University of Denmark and Emineral A/S. Scholarships were granted by Danielsen's
- 525 Foundation and Spirekassen. Värtaverket Combined Heat and Power Plant and Skærbæk
- Power Plant are acknowledged for supplying the investigated wood ashes.

References

- 528 [1] P.J.M. Monteiro, S.A. Miller, A. Horvath, Towards sustainable concrete, Nat. Mater. 16 529 (2017) 698–699. doi:10.1038/nmat4930.
- 530 [2] UN Environment, K.L. Scrivener, V.M. John, E.M. Gartner, Eco-efficient cements: Potential economically viable solutions for a low-CO2cement-based materials industry, Cem. Concr. Res. 114 (2018) 2–26.
- 533 [3] A. Neslen, The end of coal: EU energy companies pledge no new plants from 2020 (Accessed 16th of November, 2018), Guard. (2017). https://www.theguardian.com/environment/2017/apr/05/the-end-of-coal-eu-energy-companies-pledge-no-new-plants-from-2020.
- 537 [4] A.S. Frans Lamers, Marcel Cremers, Doris Matschegg, Christoph Schmidl, Kirsten Hannam, 538 Paul Hazlett, Sebnem Madrali, Birgitte Primdal Dam, Roberta Roberto, Rob Mager, Kent 539 Davidsson, Nicolai Bech, Hans-Joachim Feuerborn, Options for increased use of ash from 540 biomass combustion and co-firing, IEA Bioenergy Task 32. (2018) 1–61.
- 541 [5] R. Siddique, Utilization of wood ash in concrete manufacturing, Resour. Conserv. Recycl. 67 (2012) 27–33.
- 543 [6] M. Berra, T. Mangialardi, A.E. Paolini, Reuse of woody biomass fly ash in cement-based materials, Constr. Build. Mater. 76 (2015) 286–296. doi:10.1016/j.conbuildmat.2014.11.052.
- T. Ramos, A.M. Matos, J. Sousa-Coutinho, Mortar with wood waste ash: Mechanical strength carbonation resistance and ASR expansion, Constr. Build. Mater. 49 (2013) 343–351.
- 547 [8] R. Rajamma, R.J. Ball, L.A.C. Tarelho, G.C. Allen, J.A. Labrincha, V.M. Ferreira, Characterisation and use of biomass fly ash in cement-based materials, J. Hazard. Mater. 172 (2009) 1049–1060.
- 550 [9] S. Chowdhury, A. Maniar, O.M. Suganya, Strength development in concrete with wood ash blended cement and use of soft computing models to predict strength parameters, J. Adv. Res. 6 (2015) 907–913.
- 553 [10] A.U. Elinwa, Y.A. Mahmood, Ash from timber waste as cement replacement material, Cem. Concr. Compos. 24 (2002) 219–222.
- 555 [11] F.F. Udoeyo, H. Inyang, D.T. Young, E.E. Oparadu, Potential of wood waste ash as an additive in concrete, J. Mater. Civ. Eng. 18 (2006) 605–611.
- 557 [12] C.B. Cheah, M. Ramli, The implementation of wood waste ash as a partial cement replacement material in the production of structural grade concrete and mortar: An overview, Resour. Conserv. Recycl. 55 (2011) 669–685.

- 560 [13] R. Siddique, Wood Ash, in: Waste Mater. By-Products Concr., Springer Berlin Heidelberg, 561 Berlin, Heidelberg, 2008: pp. 303–322.
- 562 [14] R. Rajamma, R.J. Ball, L.A.C. Tarelho, G.C. Allen, J.A. Labrincha, V.M. Ferreira, Characterisation and use of biomass fly ash in cement-based materials, J. Hazard. Mater. 172 (2009) 1049–1060.
- 565 [15] G.C.H. Doudart de la Grée, M.V.A. Florea, A. Keulen, H.J.H. Brouwers, Contaminated biomass 566 fly ashes - Characterization and treatment optimization for reuse as building materials, Waste 567 Manag. 49 (2016) 96–109.
- 568 [16] J. Rosales, M. Cabrera, M.G. Beltrán, M. López, F. Agrela, Effects of treatments on biomass bottom ash applied to the manufacture of cement mortars, J. Clean. Prod. 154 (2017) 424–435. doi:10.1016/j.jclepro.2017.04.024.
- 571 [17] Danish Energy Agency, Energy Statistics 2018, 2018.
- 572 [18] R. Van Den Broek, A. Faaij, A. Van Wijk, Biomass combustion for power generation, Biomass and Bioenergy. 11 (1996) 271–281. doi:10.1016/0961-9534(96)00033-5.
- X. Li, R. Snellings, M. Antoni, N.M. Alderete, M. Ben Haha, S. Bishnoi, Ö. Cizer, M. Cyr, K.
 De Weerdt, Y. Dhandapani, J. Duchesne, J. Haufe, D. Hooton, M. Juenger, S. Kamali-Bernard,
 S. Kramar, M. Marroccoli, A.M. Joseph, A. Parashar, C. Patapy, J.L. Provis, S. Sabio, M.
 Santhanam, L. Steger, T. Sui, A. Telesca, A. Vollpracht, F. Vargas, B. Walkley, F. Winnefeld,
 G. Ye, M. Zajac, S. Zhang, K.L. Scrivener, Reactivity tests for supplementary cementitious
 materials: RILEM TC 267-TRM phase 1, Mater. Struct. Constr. 51 (2018).
- 580 [20] N.M. Sigvardsen, G.M. Kirkelund, P.E. Jensen, M.R. Geiker, L.M. Ottosen, Impact of production parameters on physiochemical characteristics of wood ash for possible utilisation in cement-based materials, Resour. Conserv. Recycl. 145 (2019) 230–240.
- 583 [21] G.M. Kirkelund, L.M. Ottosen, P.E. Jensen, P. Goltermann, Greenlandic waste incineration fly and bottom ash as secondary resource in mortar, Int. J. Sustain. Dev. Plan. 11 (2016) 719–728.
- 585 [22] EN 196, Method of testing cement Part 2: Chemical analysis of cement, (2013).
- 586 [23] EN 451, Method of testing fly ash Part 1: Determination of free calcium oxide content, (2017).
- 587 [24] EN 196, Methods of testing cement Part 6: Determination of fineness, (2018).
- 588 [25] EN 196, Method of testing cement Part 1: Determination of strength, (2016).
- 589 [26] ASTM C511-19, Standard Specification for Mixing Rooms, Moist Cabinets, Moist Rooms, and Water Storage Tanks Used in the Testing of Hydraulic Cements and Concretes, Am. Soc. Test. Mater. (2019) 1–3.
- 592 [27] K. Scrivener, R. Snellings, B. Lothenbach, A Practical Guide to Microstructural Analysis of Cementitious Materials, CRC Press, 2017.
- 594 [28] L. Pelletier-Chaignat, F. Winnefeld, B. Lothenbach, C.J. Müller, Beneficial use of limestone filler with calcium sulphoaluminate cement, Constr. Build. Mater. 26 (2012) 619–627.
- 596 [29] M. Illikainen, P. Tanskanen, P. Kinnunen, M. Körkkö, O. Peltosaari, V. Wigren, J. Österbacka, B. Talling, J. Niinimäki, Reactivity and self-hardening of fly ash from the fluidized bed combustion of wood and peat, Fuel. 135 (2014) 69–75. doi:10.1016/j.fuel.2014.06.029.
- 599 [30] ASTM International C311/C311M-13, Standard Test Methods for Sampling and Testing Fly Ash or Natural Pozzolans for Use in Portland-Cement Concrete., (2007) 204–212.
- 601 [31] EN 1015, Methods of test for mortar for masonry Part 3: Determination of consistence of fresh mortar (by flow table), (1999).

- 603 [32] ASTM International C618-15, Standard Specification for Coal Fly Ash and Raw or Calcined Natural Pozzolan for Use in Concrete, (2015).
- B. Osbæck, The influence of air content by assessing the pozzolanic activity of fly ash by strength testing, Cem. Concr. Res. 15 (1985) 53–64. doi:10.1016/0008-8846(85)90008-0.
- 607 [34] G.N. Munch-Petersen, Betonhåndbogen, Dansk Betonforening, 2013.
- 608 [35] I. Carević, M. Serdar, N. Štirmer, N. Ukrainczyk, Preliminary screening of wood biomass ashes for partial resources replacements in cementitious materials, J. Clean. Prod. 229 (2019) 1045–1064. doi:10.1016/j.jclepro.2019.04.321.
- 611 [36] R. Rajamma, L. Senff, M.J. Ribeiro, J.A. Labrincha, R.J. Ball, G.C. Allen, V.M. Ferreira, Biomass fly ash effect on fresh and hardened state properties of cement based materials, Compos. Part B Eng. 77 (2015) 1–9. doi:10.1016/j.compositesb.2015.03.019.
- B.M. Steenari, L.G. Karlsson, O. Lindqvist, Evaluation of the leaching characteristics of wood ash and the influence of ash agglomeration, Biomass and Bioenergy. 16 (1999) 119–136.
- 616 [38] P. Barnes, J. Bensted, Structure and Performance of Cements, 2nd ed., Taylor & Francis, London and New York, 2002.
- 618 [39] B.-M. Steenari, O. Lindqvist, Stabilisation of biofuel ashes for recycling to forest soil, Biomass and Bioenergy. 13 (1997) 39–50.
- 620 [40] S. van Loo, J. Koppejan, The Handbook of Biomass Combustion and Co-firing, Taylor & Francis Ltd, 2010.
- 622 [41] P. Hewlett, M. Liska, Lea's Chemistry of Cement and Concrete, 5th ed., Elsevier Science, 2019.
- T. Sebok, J. Simonik, K. Kulisek, The compressive strength of samples containing fly ash with high content of calcium sulfate and calcium oxide, Cem. Concr. Res. 31 (2001) 1101–1107.
- 625 [43] H. Justnes, Performance of SCMs Chemical and Physical Principles, in: 2nd Int. Conf. Sustain. Build. Mater., 2019.
- J. Skibsted, R. Snellings, Reactivity of supplementary cementitious materials (SCMs) in cement blends, Cem. Concr. Res. 124 (2019) 105799. doi:10.1016/j.cemconres.2019.105799.
- [45] K.L. Scrivener, B. Lothenbach, N. De Belie, E. Gruyaert, J. Skibsted, R. Snellings, A.
 Vollpracht, TC 238-SCM: hydration and microstructure of concrete with SCMs: State of the art on methods to determine degree of reaction of SCMs, Mater. Struct. Constr. 48 (2015) 835–862.
 doi:10.1617/s11527-015-0527-4.
- 633 [46] S. Donatello, M. Tyrer, C.R. Cheeseman, Comparison of test methods to assess pozzolanic activity, Cem. Concr. Compos. 32 (2010) 121–127.
- S. Ramanathan, H. Moon, M. Croly, C. Chung, P. Suraneni, Predicting the degree of reaction of supplementary cementitious materials in cementitious pastes using a pozzolanic test, Constr. Build. Mater. 204 (2019) 621–630. doi:10.1016/j.conbuildmat.2019.01.173.
- 638 [48] S. Donatello, A. Freeman-Pask, M. Tyrer, C.R. Cheeseman, Effect of milling and acid washing 639 on the pozzolanic activity of incinerator sewage sludge ash, Cem. Concr. Compos. 32 (2010) 640 54–61. doi:10.1016/j.cemconcomp.2009.092.
- 641 [49] R. Kalina, S. Al-Shmaisani, R.D. Ferron, M.C.G. Juenger, False Positives in ASTM C618 Specifications for Natural Pozzolans, ACI Mater. J. 116 (2019) 165–172.
- E. Berodier, K. Scrivener, Understanding the filler effect on the nucleation and growth of C-S-H, J. Am. Ceram. Soc. 97 (2014) 3764–3773. doi:10.1111/jace.13177.
- 645 [51] P. Lawrence, M. Cyr, E. Ringot, Mineral admixtures in mortars Effect of inert materials on short-

term hydration, Cem. Concr. Res. 33 (2003) 1939–1947.

[52] C.R. Shearer, The Productive Reuse of Coal, Biomass and the Productive Reuse of Coal, Biomass and Co-Fired Fly Ash, Georgia Institute of Technology, 2014.

[53] N.N.N. Yeboah, C.R. Shearer, S.E. Burns, K.E. Kurtis, Characterization of biomass and high carbon content coal ash for productive reuse applications, Fuel. 116 (2014) 438–447.

Table 1. Consistency measured by flow table. The value SP to obtain REF are calculated via linear interpolation of the measured flow values in order to obtain a flow value equal to the average flow value of REF.

	SP	Measure 1	Measure 2	Measure 3	Measure 4	Average	SP to obtain REF
	[g]	[mm]	[mm]	[mm]	[mm]	[mm]	[g]
REF no. 1	0	173.0	176.0	176.2	175.0	174.3	
REF no. 2	0	167.0	175.6	175.5	176.2	174.3	-
WA1 no. 1	0.5	148.8	146.2	146.9	144.8	146.7	
WA1 no. 2	1.0	183.8	184.6	177.1	178.7	181.0	1.2
WA1 no. 3	2.4	206.7	201.9	206.5	207.0	205.1	
WA1-W no. 1	0.6	148.8	148.1	143.0	152.3	147.8	
WA1-W no. 2	1.0	169.1	173.7	166.8	159.1	167.2	1.2
WA1-W no. 3	1.5	182.8	194.2	181.6	191.1	187.4	
WA2 no. 1	0.5	172.2	178.9	171.6	166.5	172.3	
WA2 no. 2	0.8	183.6	182.5	179.8	185.1	182.7	0.6
WA2 no. 3	1.0	184.9	189.0	177.4	181.0	183.1	
WA2-W no. 1	0.5	174.0	172.4	164.4	164.0	166.9	
WA2-W no. 2	0.8	175.4	164.2	159.8	173.5	168.2	0.8
WA2-W no. 3	1.0	185.2	181.9	184.6	182.4	183.5	

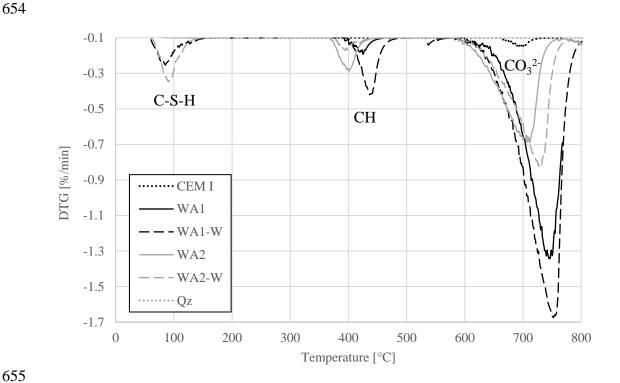


Figure 1. Differential thermosgravimetric (DTG) curves for unhydrated materials. C-S-H: Calium silicate hydrate, CH: Portlandite.

Table 2. Chemical composition (%) of CEM I, WA1, WA1-W, WA2, WA2-W, and Qz. ± defines the standard deviation. ND: not determined.

	CEM I	WA1	WA1-W	WA2	WA2-W	Qz
SiO ₂	19.7	11.0	12.7	23.8	26.5	99.4
Al_2O_3	5.4	2.4	3.0	5.6	6.3	0.10
Fe_2O_3	3.8	2.9	3.2	3.1	3.3	0.03
CaO	64.1	53.6	65.0	44.7	45.0	0
MgO	1.0	4.2	5.8	4.1	4.4	0
K_2O	0.4	14.6	4.4	7.6	5.7	0
Na ₂ O	0.3	1.0	1.0	0.8	0.9	0
P_2O_5	0.3	2.9	3.8	3.8	4.2	0
SO_3	3.2	5.4	1.3	6.1	3.7	0
CaO, free	ND	12.3	5.9	6.4	1.6	0
Cl-	0.0 ± 0.0	0.8 ± 0.0	0.0 ± 0.0	0.4 ± 0.0	0.0 ± 0.0	0.0 ± 0.0
SO_4 -2	0.0 ± 0.0	3.8 ± 0.0	0.0 ± 0.0	1.8 ± 0.0	0.0 ± 0.0	0.0 ± 0.0
LoI, 950°C	1.9 ± 0.0	15.0 ± 0.1	19.6 ± 0.1	16.2 ± 0.3	19.7 ± 0.1	0.0 ± 0.0
Unburned carbon	-	≥ 1.0	≥ 1.0	5.7	6.7	-

Table 3. Physical characteristics of CEM I, WA1, WA1-W, WA2, WA2-W, and Qz. ± defines the standard deviation.

	CEM I	WA1	WA1-W	WA2	WA2-W	Qz
pН	12.8 ± 0.0	13.0 ± 0.1	12.5 ± 0.0	12.7 ± 0.1	12.5 ± 0.2	7.7 ± 0.1
Conductivity (mS m ⁻¹)	18.1 ± 0.2	76.6 ± 0.4	11.1 ± 0.1	29.0 ± 1.3	11.9 ± 0.2	40.0 ± 2.5
Particle density (kg/m ³)	3180	2740	2640	2710	2650	2650



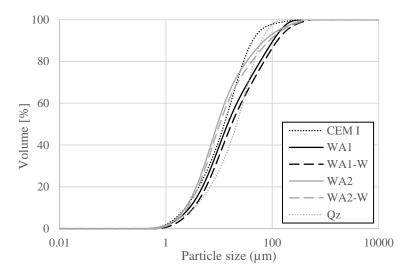


Figure 2. Particle size distribution of CEM I, WA1, WA1-W, WA2, WA2-W, and Qz, determined by laser diffraction.

	Alite	Belite-β	C_3A	Ferrite	Portlandite	Lime	Quartz	Calcite	Sylvite	Arcanite	Anhydrite
CEM I	XXX	Х	X	X	-	•					X
WA1					X	X	XX	XXX		X	
WA1-W					X		XX	XXX			
WA2					X	X	XXX	XX	X		X
WA2-W					X		XXX	XXX			
Qz							xxxx				

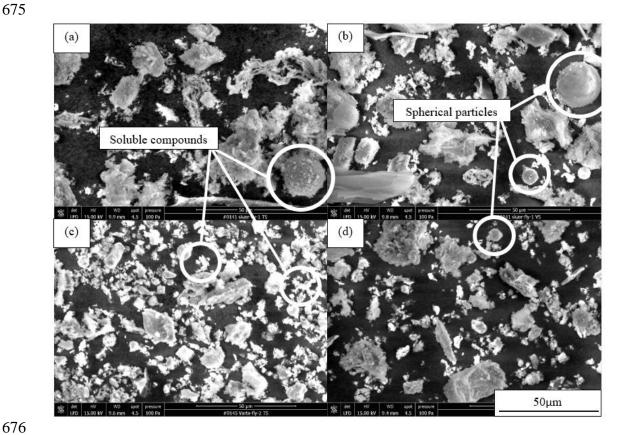


Figure 3. Grain morphology of (a) WA1, (b) WA1-W, (c) WA2 and (d) WA2-W.

Table 5. Measured compressive strength, air content, compressive strength normalised to an air content of 2 vol% by the use of Bolomeys equation [34] and SAI calculated according to ASTM C311/C311M-13 [30].

	Compressive strength (MPa)	SD	Air content (% vol)	Normalised compressive strength (MPa)	SAI
M7-REF	59.5	3.6	1.1	57.5	-
M7-20%WA1	44.1	0.7	1.0	42.6	0.74
M7-20%WA1-W	50.4	1.1	1.1	46.7	0.81
M7-20%WA2	50.3	0.7	1.0	48.7	0.85
M7-20%WA2-W	49.3	0.4	1.1	46.04	0.80
M7-20%Qz	45.9	0.9	1.0	44.1	0.77
M28-REF	69.9	1.8	1.1	64.7	-
M28-20%WA1	51.1	1.3	1.0	49.4	0.76
M28-20%WA1-W	59.3	2.7	1.1	55.0	0.85
M28-20%WA2	59.4	1.4	1.0	57.4	0.89
M28-20%WA2-W	57.0	0.7	1.1	53.24	0.82
M28-20%Qz	57.2	1.4	1.0	54.9	0.85

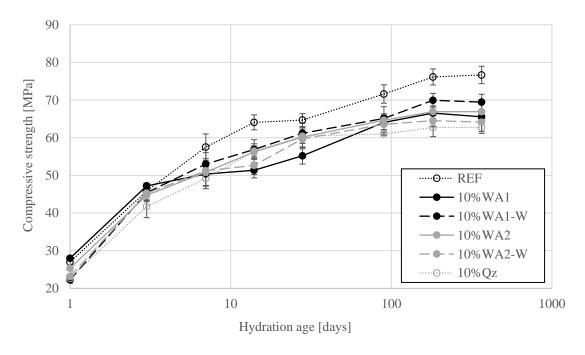


Figure 4. Development of compressive strength of REF and mortars with 10% replacements of cement with WA1, WA1-W, WA2-W, and Qz after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20°C. The compressive strength normalised to an air content of 2 vol% by the use of Bolomeys equation [34]. The error bars defines the standard deviation.

 Table 6. Measured compressive strength, air content and compressive strength normalised to an air content of 2 vol% by the use of Bolomeys equation [34].

	e strength		(%vol)	strength
	Compressive (MPa)	SD	Air content (%vol)	Normalised compressive (MPa)
M1-REF	27.9	0.6	1.1	27.0
M1-10%WA1	30.1	0.7	0.1	28.0
M1-10%WA1-W	23.2	1.0	0.9	22.3
M1-10%WA2	26.1	0.7	1.2	25.3
M1-10%WA2-W	24.8	0.9	0.2	23.2
M1-10%Qz	23.8	0.9	0.8	22.7
M3-REF	47.4	1.6	1.1	45.8
M3-10%WA1	50.8	0.8	0.1	47.2
M3-10%WA1-W	47.4	2.2	0.9	45.4
M3-10%WA2	46.1	1.7	1.2	44.7
M3-10%WA2-W	48.4	1.9	0.2	45.3
M3-10%Qz	43.8	3.1	0.8	41.7
M7-REF	59.5	3.6	1.1	57.5
M7-10%WA1	54.2	3.5	0.1	50.4
M7-10%WA1-W	55.3	3.2	0.9	53.0
M7-10%WA2	52.5	3.7	1.2	50.9
M7-10%WA2-W	54.8	1.2	0.2	51.2
M7-10%Qz	51.6	2.9	0.8	49.2
M14-REF	67.1	2.1	1.1	64.1
M14-10%WA1	55.3	2.2	0.1	51.4
M14-10%WA1-W	59.4	2.7	0.9	56.9
M14-10%WA2	58.0	1.6	1.2	56.2
M14-10%WA2-W	56.4	2.6	0.2	52.7
M14-10%Qz	59.2	2.0	0.8	56.4
M28-REF	69.9	1.8	1.1	64.7
M28-10%WA1	59.4	2.4	0.1	55.2
M28-10%WA1-W	63.9	2.8	0.9	61.2
M28-10%WA2	62.1	2.7	1.2	60.2
M28-10%WA2-W	63.9	2.8	0.2	59.7
M28-10%Qz	63.6	1.8	0.8	60.4
M90-REF	74.1	2.5	1.1	71.6
M90-10%WA1	68.9	2.1	0.1	64.1
M90-10%WA1-W	68.0	3.2	0.9	65.1
M90-10%WA2	66.7	1.1	1.2	64.6
M90-10%WA2-W	68.0	3.0	0.2	63.6
M90-10%Qz	64.0	0.4	0.8	61.0
M182-REF	78.8	2.2	1.1	76.1
M182-10%WA1	71.6	2.1	0.1	66.5
M182-10%WA1-W	73.0	1.9	0.9	69.9
M182-10%WA2	69.1	2.8	1.2	66.9
M182-10%WA2-W	69.1	1.7	0.2	64.5
M182-10%Qz	65.8	0.4	0.8	62.7
M365-REF	77.0	2.5	1.1	76.7
M365-10%WA1	70.5	1.4	0.1	65.6
M365-10%WA1-W	71.5	2.2	0.9	69.4
M365-10%WA2	69.1	2.4	1.2	66.9
M365-10%WA2-W	68.6	2.7	0.2	64.1
M365-10%Qz	67.9	1.6	0.8	62.7

Table 7. Crystalline phases for paste samples investigated after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20° C determined qualitatively by XRD diffraction. x: < 15%, $xx: \sim 25\%$ and $xxx: \sim 50\%$ of crystalline phases identified according to semiquantitative analysis.

	Lime	Alite	Belite-β	Ferrite	Portlandite	Ettringite	Quartz	Calcite	Monocarbonate
P1-REF	-	XX	XX	X	XX	X			- '
P1-10%WA1	х	XX	XX	X	XX	X	x	x	
P1-10%WA1-W		XX	XX	X	XX	X	X	X	
P1-10%WA2	x	XX	XX	x	XX	X	X	x	
P1-10%WA2-W		XX	XX	X	XX	X	X	X	
P1-10%Qz		XX	X	x	X	X	XX		
P3-REF	<u> </u>	XX	XX	X	XX	X		-	
P3-10%WA1		X	XX	X	XX	XX	x	x	
P3-10%WA1-W		XX	XX	X	XX	XX	X	X	
P3-10%WA2		X	XX	x	XXX	XX	X	**	
P3-10%WA2-W		X	XX	X	XX	XX	X		
P3-10%Qz		x	X	x	XX	XX	xx		
P7-REF			XX	X	XX	XX			
P7-10%WA1			XX	X	XX	XX	X	X	
P7-10%WA1-W			XX	x	XX	XX	X	X	
P7-10%WA2			X	X	XXX	XX	X		
P7-10%WA2-W			X	x	XX	XX	X		
P7-10%Qz			X	X	XX	XX	XX		
P14-REF			XX	X	XXX	XX			X
P14-10%WA1			X	X	XXX	XX	X	X	X
P14-10%WA1-W			X	x	XXX	XX	X	X	x
P14-10%WA2			X	X	XXX	XX	X	**	X
P14-10%WA2-W			X	X	XXX	XX	X		X
P14-10%Qz			X	X	XXX	XX	XX		X
P28-REF			X	X	XXX	XX			X
P28-10%WA1				••	XX	XX	X	x	X
P28-10%WA1-W			X	x	XX	XX	x	X	X
P28-10%WA2			X	X	XX	XX	X		X
P28-10%WA2-W			X	x	XX	XX	X		X
P28-10%Qz					XXX	XX	XX		X
P90-REF					XXX	XX			X
P90-10%WA1					XXX	XXX	X	X	X
P90-10%WA1-W					XXX	XX	X	X	X
P90-10%WA2					XXX	XX	X	X	X
P90-10%WA2-W					XXX	XX	X	X	X
P90-10%Qz					XXX	XX	XX		X
P182-REF					XXX	XX			X
P182-10%WA1					XXX	XXX	X	X	X
P182-10%WA1-W					XXX	XX	x	x	X
P182-10%WA2					XXX	XX	x	X	X
P182-10%WA2-W					XXX	XX	X	X	X
P182-10%Qz					XXX	XX	XX		X
P365-REF					XXX	XX			X
P365-10%WA1					XXX	XXX	X	X	X
P365-10%WA1-W					XXX	XX	x	x	X
P365-10%WA2					XXX	XXX	x	x	X
P365-10%WA2-W					XXX	XX	x	x	X
P365-10%Qz					XXX	XX	XX	-	x
- 500 2070 Q E					АЛА	7878	AA		71

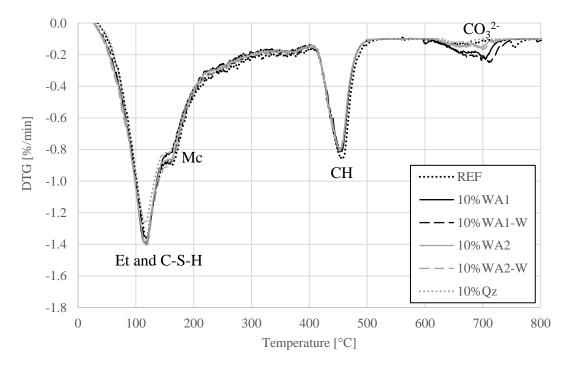


Figure 5. Differential thermosgravimetric (DTG) curves for paste samples investigated at 20°C after 28 days of hydration. Et: Ettringite, C-S-H: Calium silicate hydrate, Mc: Monocarbonate, CH: Portlandite.

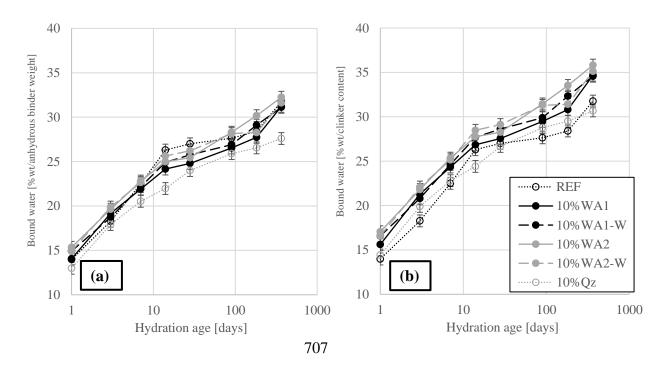


Figure 6. Quantification of bound water in paste samples investigated after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20°C. The results are normalised to the anhydrous binder (a) and clinker content (b). Standard deviations are based on three independent measurements and quantifications of the bound water of a control sample at 28 days of hydration.



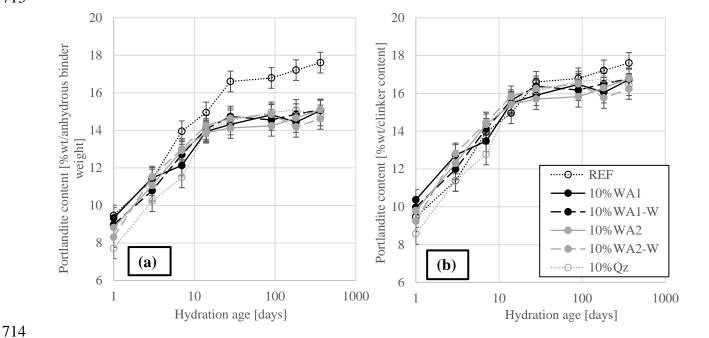


Figure 7. Quantification of portlandite in paste samples investigated after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20°C. The results are normalised to the anhydrous binder (a) and clinker content (b). Standard deviations are based on three independent measurements and quantifications of the portlandite of a control sample at 28 days of hydration.

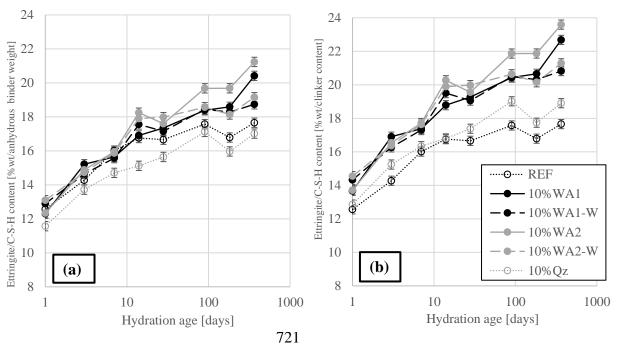


Figure 8. Quantification of ettringite/C-S-H in paste samples investigated after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20°C. The results are normalised to the anhydrous binder (a) and clinker content (b). Standard deviations are based on three independent measurements and quantifications of the ettringite of a control sample at 28 days of hydration.

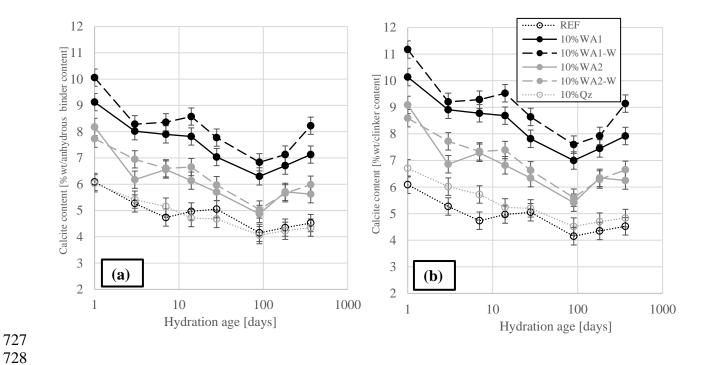


Figure 9. Quantification of calcite in paste samples investigated after 1, 3, 7, 14, 28, 90, 182 and 365 days of hydration at 20°C. The results are normalised to the anhydrous binder (a) and clinker content (b). Standard deviations are based on three independent measurements and quantifications of the calcite of a control sample at 28 days of hydration.