

# Bivariate statistics of wind farm support vessel motions while docking

Oleg Gaidai<sup>1\*)</sup>, Xiaosen Xu<sup>2)</sup>, Arvid Naess<sup>3)</sup>, Yong Cheng<sup>1)</sup>, Renchuan Ye<sup>1)</sup>, Junlei Wang<sup>4)</sup>

<sup>1)</sup> Jiangsu University of Science and Technology, Zhenjiang, China

<sup>2)</sup> Florida Institute of Technology, USA

<sup>3)</sup> Norwegian University of Science and Technology, Norway

<sup>4)</sup> School of Mechanical and Power Engineering, Zhengzhou University, China

## 1 Abstract

Robust prediction of extreme motions during wind farm support vessel (WFSV) operation is an important safety concern. In particular, it is important to study safety of operation in random sea conditions during WFSV docking against the wind tower, while workers are able to get on to the tower. Docking is performed by thrusting the vessel fender against the wind tower (the alternative docking maneuver by hinging is not studied here). In this paper, the finite element software AQWA has been used to analyze the vessel response due to hydrodynamic wave loads, acting on a specific maintenance ship under actual sea conditions. Excessive motions may occur during certain sea conditions, posing a risk to the crew transfer operation. The authors have primarily focused on the statistical analysis rather than the dynamics of the problem.

This paper presents a novel method for estimating bivariate statistics, based on Monte Carlo simulations (or measurements if available). The bivariate average conditional exceedance rate (ACER2D) method is briefly outlined. The ACER2D method offers an accurate estimation of bivariate statistics, utilizing the available data efficiently. Two dimensional probability contours, corresponding to large return periods, are obtained by the ACER2D method. Based on the overall performance of the presented method, it is seen that the ACER2D method provides an efficient and accurate prediction of extreme return period contours.

The described approach may serve as a useful tool for vessel design, facilitating optimization of boat parameters in order to minimize excessive vessel motions.

## Keywords

Wind farm support vessel (WFSV); docking; Monte Carlo; bivariate distribution; offshore wind turbine; wind energy.

## 2 Introduction

The offshore renewable wind energy market is expected to have a dramatic growth in the next decades. Operational costs and safety issues related to wind farm maintenance are among the most crucial issues for the renewable energy sector. For an onshore wind farm, the maintenance tasks can be carried out almost at any time when the weather conditions are not extreme. On the other hand, for an offshore wind farm (OWF) operation, a number of issues like transportation of maintenance personnel, equipment, spare parts, and access to the wind turbines from service vessels are restricted by sea conditions. Down time of wind turbines due to waiting for weather windows is one of the major contributors to the loss of electricity production and hence income. Innovative vessel and access concepts can help to

\*) Corresponding author: Oleg Gaidai, tel. +8618852803218; email: o\_gaidai@just.edu.cn

46 reduce the weather restrictions on the access operation and therefore decrease the downtime for offshore wind  
47 turbines. The latter implies that certain savings in costs and increase in operational safety can be obtained by  
48 optimizing vessel characteristics, especially on the design stage.

49 An accurate modeling of the WFSV extreme motions during docking against the wind tower should be conducted in  
50 order to evaluate operational risks. Relatively few studies have been done in the latter direction, see e.g.  
51 Bondarenko(2015); Wu (2014) for the docking operation numerical analysis of the WFSV with a simple fender. See  
52 also Zhang et al . (2014a) for a study of operation modelling of a WFSV using stochastic activity networks.  
53

54

55 A certain amount of research work has been done to study the motions of WFSV in waves, see e.g. Wu (2014);  
56 Zhang et al. (2014a); Zhang et al. (2014b); Naess and Moan (2013); Price and Bishop (1974); Sandvik (2010);  
57 Phillips et al. (2014); Phillips et al. (2015) for the WFSV docking operation with a fender system. Analyzing  
58 complex vessel motions during step-across-fender transfer are of importance for safety of operations, see Phillips et  
59 al. (2014); Phillips et al. (2015). Statistics of extreme motions is a key issue for the safety and reliability analysis  
60 during offshore operations; see Bondarenko(2015); Naess and Moan (2013); Price and Bishop (1974) for the basics  
61 of the probabilistic theory for ships and offshore structures.

62 It can be concluded from a study of the existing literature that there is no preferred methodology for prediction of the  
63 WFSV extreme motion statistics. Vessel heave and pitch are among of the most critical vessel motion types, having  
64 an important effect on the transfer safety during docking operation. This paper studies extreme value statistics of  
65 WFSV combined heave and pitch motions, during docking against a wind tower in random sea conditions.  
66

67

68 The aim of this paper is to develop a simple and general Monte Carlo (MC) based method, which is able to tackle all  
69 nonlinear effects without simplifications, except for those inherent in the hydrodynamic model itself. This goal has  
70 been achieved by combining standard MC simulations with a CPU time-saving extrapolation scheme Naess and  
71 Karpa (2015); Karpa and Naess (2015); Gaidai et al. (2016).

72 As opposed to univariate statistical methods, the ACER2D method takes into account bivariate correlation, which  
73 may reflect important coupling effects. Due to only partial correlation between the vessel heave and pitch angles,  
74 application of the multivariate, or bivariate in the simplest case, extreme value theory is of practical interest. The  
75 bivariate ACER2D method is not the only approach that serves estimating bivariate statistics, see as example  
76 Heffernan and Tawn (2004); Ewans (2014) for the IFORM (inverse first order reliability method), or SORM (second  
77 order reliability method), see e.g. Zhao and Ono (1999). An advantage of the ACER2D method is that it does not rely  
78 on asymptotic assumptions and do not exclude non-linearities.  
79

80

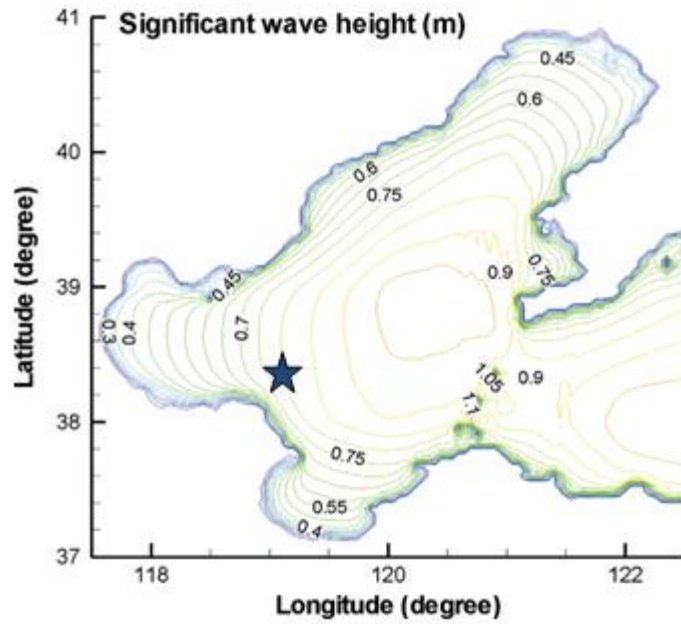
81

82

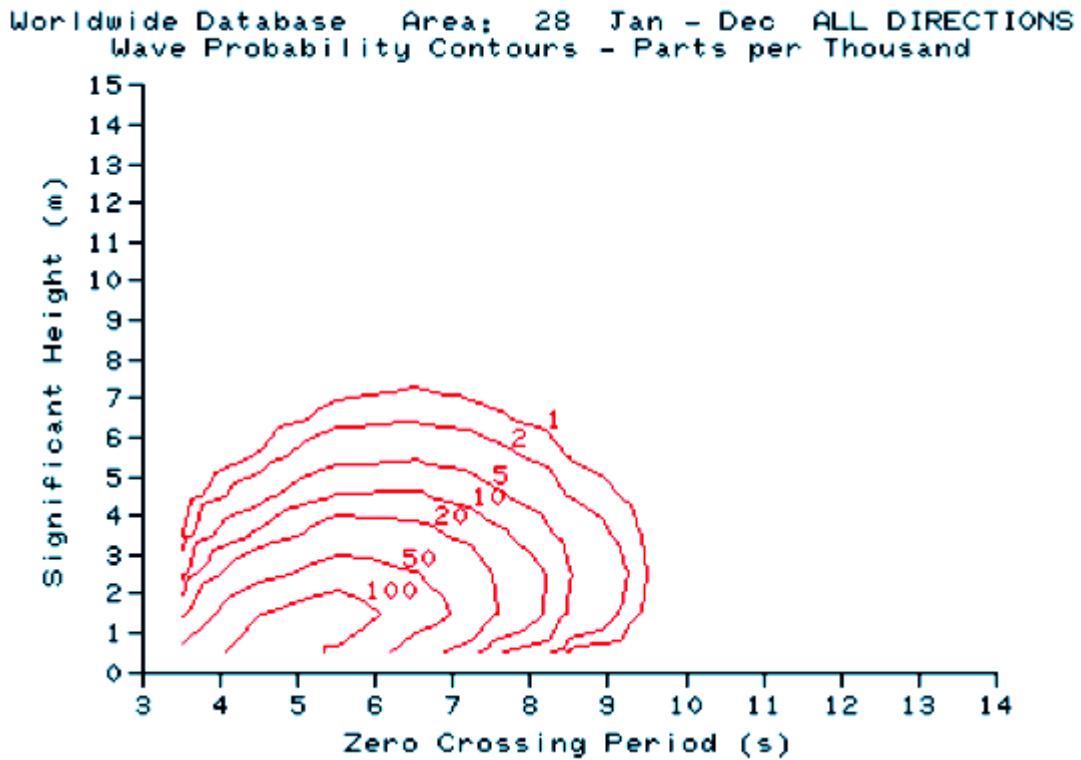
83

### 81 **3 Wave statistics**

82 Satellite based global wave statistics was used to obtain the wave scatter diagram in the Bohai bay area, where the  
83 wind farm is located. Specifically, commercial Global Wave Statistics Online data  
84 <http://www.globalwavestatisticsonline.com/> was purchased. Table 1 presents directional probabilities of wind and  
85 waves in Bohai bay <http://www.globalwavestatisticsonline.com/>, Lv et al. (2014) averaged over the whole year  
86 (seasonal variations are averaged). Fig. 1 presents annually averaged spatial distributions of wave height in Bohai  
87 bay Lv et al. (2014) ; Zhang et al. (2018).  
88



89  
 90 **Fig. 1** Annually averaged spatial distribution of wave height in Bohai Sea, Lv et al. (2014). **Star indicates**  
 91 **WFSV location.**  
 92



93  
 94 **Fig. 2** Environmental contour lines for Bohai bay area from <http://www.globalwavestatisticsonline.com/>.  
 95 **Contour numbers: fare parts per thousand.**

96 Fig. 2 presents environmental contour lines for the Bohai bay area, averaged annually and averaged over all eight  
 97 wave directions. Due to the limitation of WFSV operating in the sea with significant wave height  $H_s$  larger than 1.6  
 98 m, sea states with wave height over 1.6 m were removed from the measured sea state dataset. Thus statistics has been  
 99 made conditional on the operational safety requirement. Subsequently, an approximate sea state scatter diagram was  
 100 estimated based on environmental contour lines data from Fig. 2. **The modified scatter diagram (once the wave**  
 101 **heights larger than 1.6m have been removed) has not been presented in this paper due to its low resolution,**  
 102 **justifying it by the fact that the aim of the proposed paper was an illustration of statistical technique, and not**

103 an accurate in situ engineering estimate. The zero crossing period from Fig. 2 is approximately linearly related to  
 104 the spectral peak wave period  $T_p$ , see DNV rule DNV-RP-H103 (2011).

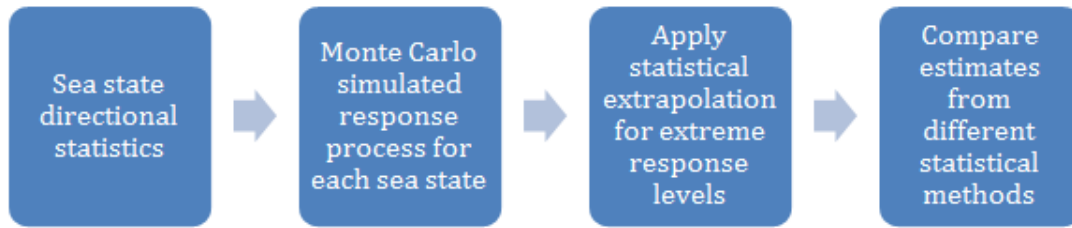
105 **Table 1 Directional probabilities of wind and waves in Bohai bay** <http://www.globalwavestatisticsonline.com/>.

Direction	All Year
North-East	14.89 %
East	11.11 %
South-East	10.01 %
South	13.15 %
South-West	7.65 %
West	8.16 %
North-West	14.21 %
North	20.82 %

106  
 107 Fig. 2 presents an environmental contour plot averaged over the eight wave directions, as authors did not want to  
 108 present eight different directional environmental contour plots. The separate wave statistics per each direction was  
 109 available from (<http://www.globalwavestatisticsonline.com/>).

110  
 111 A three hour stationary storm simulation was run for each sea state of each directional sea state  $(H_s, T_p)$ ,  
 112 approximately estimated based on Fig. 2. For each particular directional storm at a certain significant wave height the  
 113 total of 8 random sea realizations (random seeds) have been generated, amounting to  $T_{tot} = 24$  hours direct Monte  
 114 Carlo simulation for each sea state, the latter  $T_{tot}$  is typical for offshore engineering applications, see e.g. Karpa  
 115 (2015); Naess et al (2007). As an overview, Fig. 3 presents a flow chart for the methodology applied in this paper.  
 116 The significant wave height in the simulation ranged from 0.6 m to 1.6 m with the bin size of 0.2 m. First 600 s  
 117 simulation results have been discarded due to initial transient effects.

118



119  
 120 **Fig. 3 Flow chart for the described methodology.**

121  
 122 The random stationary sea states are specified by a JONSWAP wave spectrum, that is, the one-sided power spectral  
 123 density (PSD) of the wave elevation  $\eta(t)$ , denoted by  $S_{\eta}^{+}(\omega)$ , is given as follows ( $\omega > 0$ )

124 
$$S_{\eta}^{+}(\omega) = \frac{\alpha g^2}{\omega^5} \exp\left\{-\frac{5}{4}\left(\frac{\omega_p}{\omega}\right)^4 + \ln \gamma \exp\left[-\frac{1}{2\sigma^2}\left(\frac{\omega}{\omega_p} - 1\right)^2\right]\right\}$$

125 with  $g = 9.81 \text{ m/s}^2$ ,  $\omega_p$  denoting the peak frequency in rad/s; and  $\alpha$ ,  $\gamma$  and  $\sigma$  being parameters related to the  
 126 spectral shape;  $\sigma = 0.07$  when  $\omega \leq \omega_p$ ,  $\sigma = 0.09$  when  $\omega > \omega_p$ . For the Bohai bay, the parameter  $\gamma$  is  
 127 chosen to be equal to 3.3, see Wang et al. (2012); the parameter  $\alpha$  is determined from the following empirical  
 128 relationship  $\alpha = 5.06\left(\frac{H_s}{T_p^2}\right)^2(1 - 0.287 \ln \gamma)$ , see DNV rules DNV-RP-C205 (2010); DNV-RP-H103 (2011).  $H_s$   
 129 is the significant wave height, and  $T_p = 2\pi/\omega_p$  is the spectral peak wave period.

130

131

#### 132 **4 Modelling of vessel motions during docking in waves, using AQWA**

133

134 AQWA software has been developed by ANSYS Inc. It is an efficient tool for simulating nonlinear wave loading on

135 both floating and fixed rigid bodies, as well as nonlinear vessel responses, see AQWA users manual (2013). The  
 136 latter is done by employing three-dimensional radiation/diffraction theory in regular waves in the frequency domain.  
 137 To account for coupling between floating body dynamics and structural response, the AQWA software employs  
 138 specific modules such as AQWA (diffraction model), AQWA WAVE (linking program), and ANSYS (structural  
 139 finite element analysis).

140 Unidirectional or multidirectional second order drift forces can be evaluated by the far-field, near field solution, or  
 141 full quadratic transfer function (QTF) matrix. Free floating hydrostatic and hydrodynamic analyses in the frequency  
 142 domain can be performed as well, see AQWA users manual (2013).

143 The real-time motion of a floating body while operating in regular or irregular waves can be accurately simulated  
 144 with AQWA, with nonlinear Froude-Krylov and hydrostatic forces being estimated under instantaneous incident  
 145 wave surface. The real-time motion of a floating body while operating in irregular waves, can be simulated under  
 146 first- and second-order wave excitations.

147  
 148



149  
 150

151 **Fig. 4 Fender docking operation of the maintenance ship against wind tower.**

152 Fig. 5 on the left corresponds to Fig. 4 and it shows the corresponding AQWA finite element model of the wind  
 153 turbine tower and maintenance ship during fender docking against wind tower. As noted above, this paper considers  
 154 the vessel thrusts with fender against the wind tower, as opposed to the vessel hinging to wind tower.

155

156 **Table 2 Some WFSV vessel parameters.**

<i>Parameter</i>	<i>Value</i>	<i>Unit</i>
Length between perpendiculars	18.0	m
Breadth	6.5	m
Draft	1.3	m
Displacement tonnage	42.0	tones
Longitudinal center of gravity	8.0	m
Vertical center of gravity	2.5	m

157  
 158  
 159  
 160

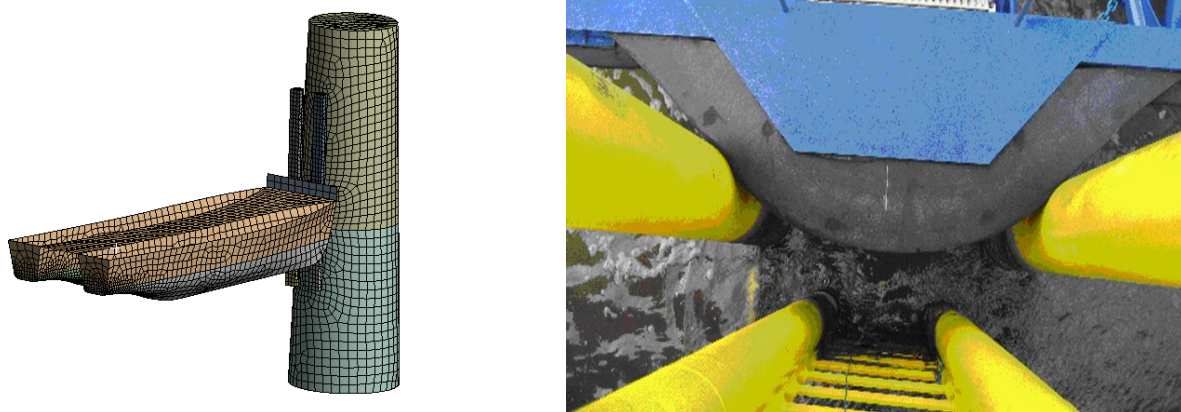
Table 2 presents some basic WFSV vessel parameters. Both 1st and 2nd order forces are computed in AQWA by utilizing linear transfer function (LTF) and quadratic transfer function (QTF).

161 Fig. 5 on the right presents a top view of the fender during docking. Several different access/docking devices are  
162 typically employed between a service vessel and a wind turbine. In this paper the simplest type is modelled, namely a  
163 fender made of rubber or other materials. The bollard push force from the propulsion system keeps the ship bow  
164 attached to the wind turbine tower (foundation) while the personnel access the ladder on the tower from the bow.

165 The punctual contact or friction force tends to restrict relative translational motions (surge, sway and heave) to  
166 almost zero, but does not assure that the relative angular motion (roll, pitch and yaw) between the two elements of  
167 the contact be zero. This is the way the roll motion of the maintenance ship is different to that of the wind turbine  
168 tower. The friction moment was neglected in the roll motion calculation. The fender friction force  $F_f$  on the  
169 contacting surface was given by  $F_f = \mu T$ , where  $\mu$  is the friction coefficient, and  $T$  is the normal compression  
170 reaction force.

171 In this paper the AQWA-Drift time-domain simulation approach was adopted to model the vessel motions and the  
172 non-linear reaction forces between the fender and the wind turbine tower. The fender damping force  $F_d$  was only  
173 applied in the direction of the fender compression force. A linear material damping model is adopted in AQWA,  
174 namely  $F_d = \beta K_f \frac{dL_d}{dt}$ , with  $\beta$  being the damping coefficient and  $K_f$  being the fender stiffness, and  $L_d$  being the  
175 distance between the two contacting points of the fender, see AQWA theory manual (2013) for details.

176 Fig. 6 presents an example of the roll moment QTF in dimensional form, per unit length squared, in the beam sea:  
177 difference frequency (left) and sum frequency (right). As expected, the sum frequency QTF has sharp peaks, while  
178 the difference frequency QTF, which is responsible for vessel slow motions, is much less 'peaked'.



179  
180 **Fig. 5 Coupled panel model of the wind turbine tower and maintenance ship (on the left); a top view of fender**  
181 **during docking (on the right).**

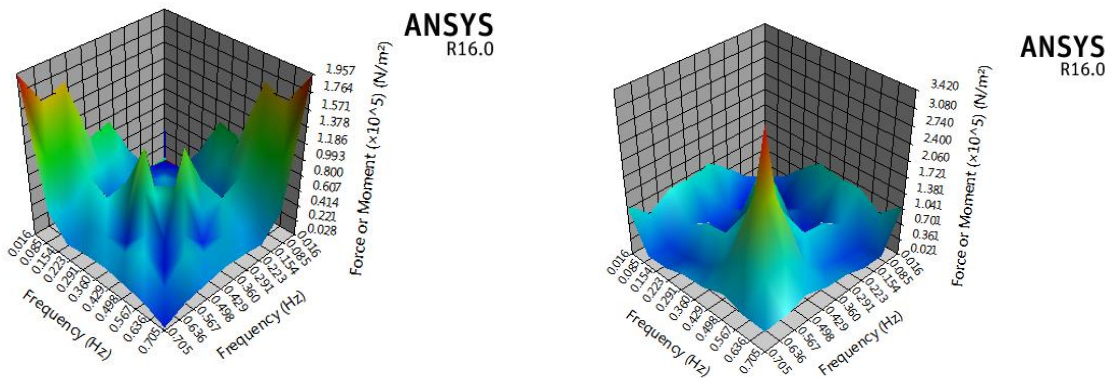
182 The numerical simulation was based on AQWA-Line and AQWA-Drift models are utilized to perform the  
183 frequency-domain analysis and the time-domain analysis respectively. Thus AQWA-Line tool was used to generate  
184 vessel LFT and QTF transfer functions. Vessel current and wind force coefficients at different incident angles were  
185 calculated using commercial ANSYS Fluent software and applied in the AQWA-Drift model, see AQWA theory  
186 manual (2013).

187 The fender and its wind tower interaction force were simulated in the AQWA-Drift (time-domain) model and the  
188 diameter of the fender was 0.2m. Bollard push force was put at the aft of the support vessel and its value was set to  
189 80kN. Table 2 presents some basic WFSV vessel parameters.

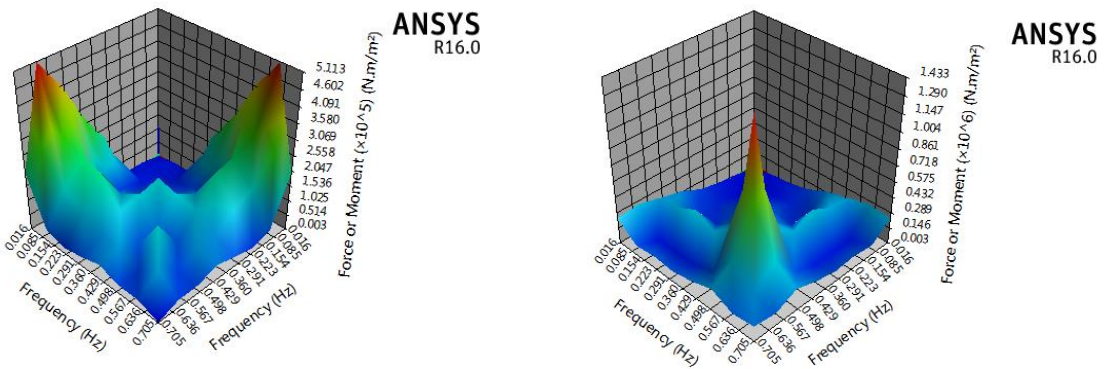
190  
191 The size of the mesh was chosen smaller than 1/7 of the shortest wave length; the time step was chosen smaller than  
192 1/10 of the smallest wave period, in accordance with AQWA requirements. The mesh size and time step  $\Delta t$  were  
193 chosen in accordance with AQWA convergence criteria, to ensure accuracy of simulated results, particularly vessel  
194 motions in waves. **For the sake of assuring convergence, time step  $\Delta t$  has been halved for the one selected sea state,**  
195 **and numerical results have been found convergent with original simulation.**

196  
197 Finally, a brief discussion follows regarding numerical modelling of physical phenomena, such as for example ship  
198 maneuvering, occurring during docking of the vessel to the platform. Employing a CFD solver to solve Navier-  
199 Stokes equations is not unique, but one possible approach to simulate ship motions. For an overview of state-of-the-  
200 art of ship motion modeling, see Reed and Beck(2016). However, including all relevant hydrodynamic components,  
201 such as e.g. lifting/ maneuvering forces and viscosity effects, was not a practical approach for a long-term simulation,  
202 since it would require prohibitively large computational costs. Besides, AQWA is computationally less expensive  
203 than, say solving directly the full Navier-Stokes equations. Note that AQWA is based on potential flow theory and it  
204 takes non-linear hydrodynamic forces as well as coupled multi-body interaction into consideration. Viscous damping  
205 coefficients were added in AQWA in order to compensate for the deficiency of the potential flow theory. Wind and  
206 current drag coefficients were also calculated by using ANSYS Fluent and added to AQWA.

207 The purpose of the maintenance ship is to transfer the personnel and equipment, and the process of transferring  
 208 usually takes only minutes. Therefore the maneuvering is not of importance in this study, due to short duration of  
 209 transferring.  
 210



211  
 212 **Fig. 6 Example of heave QTF amplitude: difference frequency (left) and sum frequency (right).**



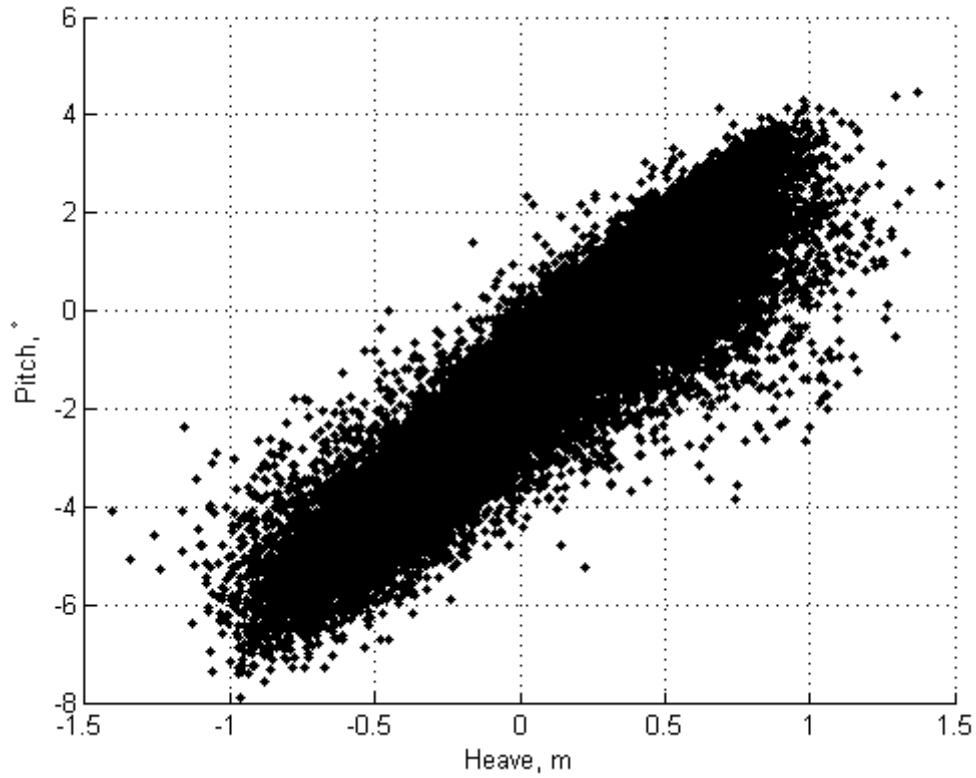
213  
 214 **Fig. 7 Example of pitch angle QTF amplitude: difference frequency (left) and sum frequency (right).**

215 Fig. 6 presents an example of the heave QTF (quadratic transfer function) amplitude in the 45° directional sea, Fig. 7  
 216 presents an example of the pitch angle QTF; difference frequency (left) and sum frequency (right). The numerical  
 217 simulation utilized AQWA-Line and AQWA-Drift models to perform frequency-domain and time-domain analysis  
 218 respectively. The 1<sup>st</sup> order and 2<sup>nd</sup> forces have been computed in AQWA. The numerical simulation was based on  
 219 AQWA-Line and AQWA-Drift models to perform the frequency-domain analysis and the time-domain analysis,  
 220 respectively. Thus AQWA-Line FEM tool was used to generate vessel LTF (linear transfer function) and QTF  
 221 transfer functions. Vessel current and wind force coefficients at different incident angles were calculated in Fluent  
 222 and applied in the AQWA-Drift model.

223 The fender along with its wind tower interaction force were simulated in the AQWA-Drift (time-domain) model. The  
 224 size of the mesh was chosen smaller than 1/7 of the shortest wave length; the time step was chosen smaller than 1/10  
 225 of the smallest wave period, in accordance with AQWA requirements. The relevant experimental validation of  
 226 AQWA numerical results, see Xu et al. (2019a); Xu et al. (2019b) discussing experimental results on the FPSO  
 227 (floating production storage and offloading unit) vessel, insures that AQWA is a proper and accurate tool to study  
 228 complex vessel motions. Both WFSV transfer operation and FPSO offloading operation exhibit similar nature of  
 229 combined wave motions and structural interactions.

230  
 231  
 232  
 233 **5 Statistical approach – the ACER2D method**

234  
 235 The Averaged Conditional Exceedance Rate (ACER2D) method has been applied to analyse vessel motion data in  
 236 order to assess the extreme bivariate heave and pitch statistics. The major advantage of the ACER2D method is that  
 237 it takes into account the full non-stationary data set, and it allows a pre-asymptotic behaviour, rather than relying on  
 238 an ad-hoc asymptotic assumption. Note that both the stochastic processes (heave and pitch in this paper) are required  
 239 to be synchronous in time, which allows an in-depth look into the long term correlation statistics and coupling  
 240 effects. The brief description of the bivariate ACER2D method is given in Appendix, for more details see Naess and



242

243 **Fig. 8 Vessel COG heave (meters) versus synchronous pitch angle (degrees).**

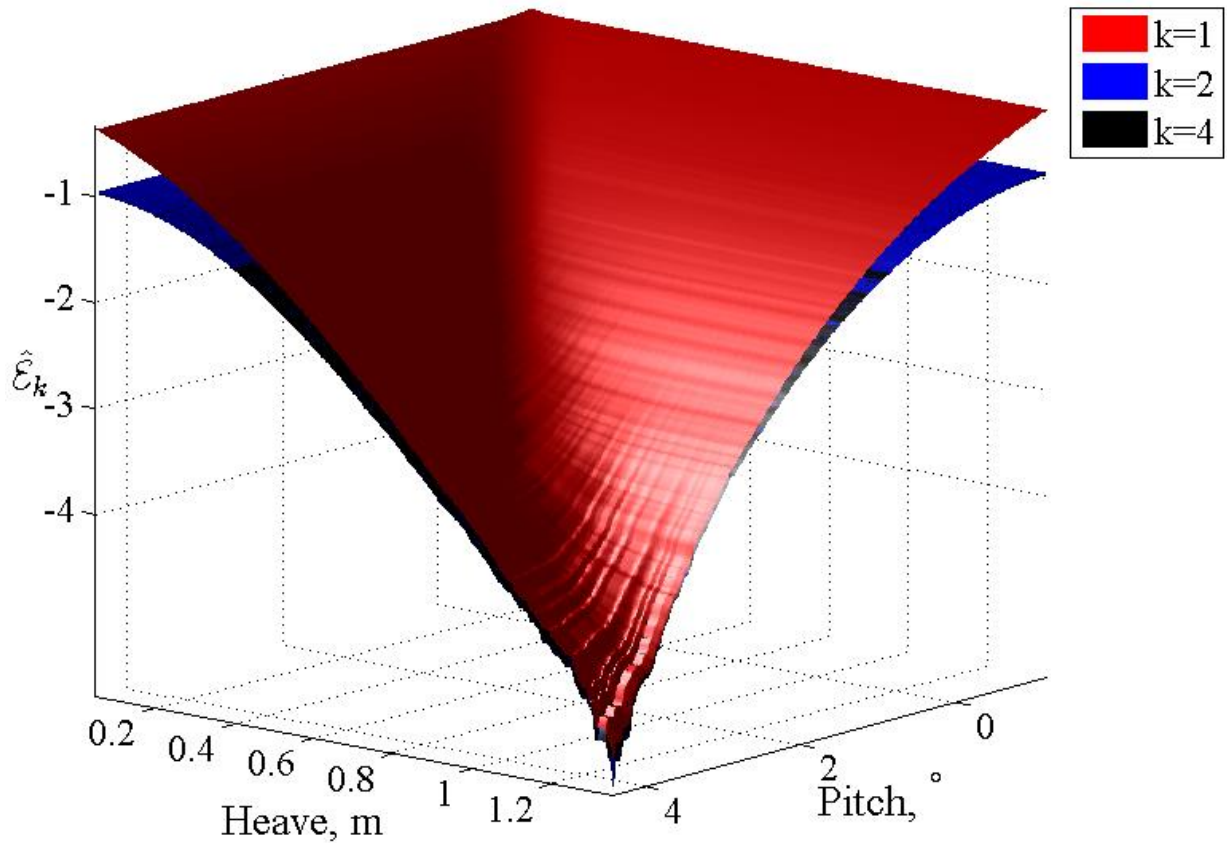
244

245 Fig. 8 illustrates the vessel's center of gravity (COG) heave displacement versus the synchronously recorded pitch  
246 angle. This would represent a sample of the long term bivariate distribution of heave and pitch, which clearly  
247 displays a significant correlation between the two random variables.

248

249



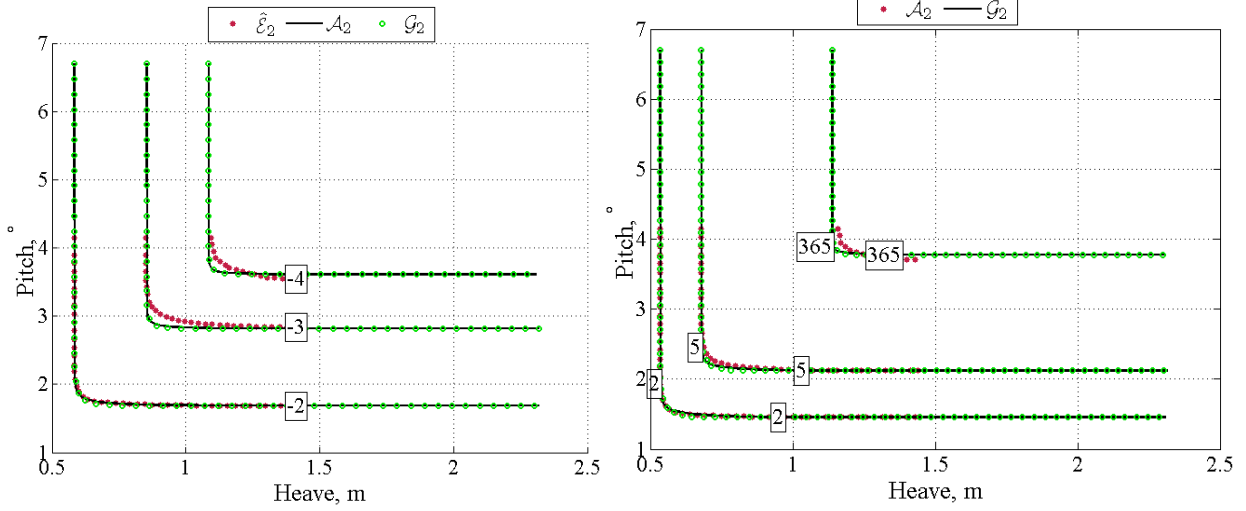


250  
 251 **Fig. 9 Comparison between ACER2D surfaces for different degrees of conditioning.  $\hat{E}_k(\xi, \eta)$  functions are**  
 252 **plotted on a decimal logarithmic scale;  $\xi$  is heave,  $\eta$  is pitch angle.**

253  
 254 Fig. 9 presents the empirically estimated bivariate ACER2D functions  $\hat{E}_k(\xi, \eta)$  for different conditioning values of  $k$   
 255 on a decimal logarithmic scale.  $\hat{E}_k(\xi, \eta)$  with  $k = 1$  is represented by the upper surface, while the following surfaces  
 256 that converge in the tail for  $k \geq 2$ . As it is seen from the Fig. 9, the cross-section of the surfaces at the high level of  
 257 heave  $\xi$  gives the univariate ACER2D functions of the pitch angle  $\eta$ , while the cross-section at a high pitch angle  
 258 level represents the univariate ACER2D of the heave, respectively. Since the surfaces for  $k \geq 2$  have converged in  
 259 the tail and estimation of  $\hat{E}_2(\xi, \eta)$  is more accurate due more data available, one is to choose the function (and thus  
 260 its surface) with the degree of conditioning  $k = 2$ .

261  
 262 Fig. 10 Left presents contour lines for the optimized Asymmetric logistic (AL)  $\mathcal{A}_k(\xi, \eta)$  and optimized Gumbel  
 263 logistic (GL)  $\mathcal{G}_k(\xi, \eta)$  models, optimally fit to the empirical bivariate ACER2D function  $\hat{E}_k(\xi, \eta)$ ,  $k = 2$ , see  
 264 Appendix for GL and AL definitions. The negative numbers at the contour lines in Fig. 10 indicate probability levels  
 265 of  $P(\xi, \eta)$  from Eq. (1) in Appendix on a decimal logarithmic scale. Fig. 10 Left shows that the empirical bivariate  
 266 ACER2D surface  $\hat{E}_2$  captures quite well the high correlation between the data. It is also seen that the optimized  
 267 models  $\mathcal{G}_2$  and  $\mathcal{A}_2$  give smooth complete contours. Note that if the bivariate data would be fully correlated, the  
 268 contour lines would consist of only horizontal and vertical straight segments. Fig. 10 Left shows quite good  
 269 agreement between the estimated bivariate ACER2D and the optimized AL and GL surfaces.

270  
 271



272

273 **Fig. 10 Left: Contour plot of the empirically estimated  $\hat{\mathcal{E}}_2(\xi, \eta)$  surface (•), optimized Gumbel logistic  $\mathcal{G}_2(\xi, \eta)$**   
 274 **(◦) and optimized Asymmetric logistic  $\mathcal{A}_2(\xi, \eta)$  (—) surfaces. Negative numbers indicate probability levels on**  
 275 **a decimal logarithmic scale. Right: Contour plot of the return periods for optimized Gumbel logistic  $\mathcal{G}_2(\xi, \eta)$**   
 276 **(◦) and optimized Asymmetric logistic  $\mathcal{A}_2(\xi, \eta)$  (—) surfaces. Boxes indicate return periods in days.**

277

278 Fig. 10 Right presents contours with return periods in days, 2, 5 and 365 days respectively per contour line. Return  
 279 period of the order of years is of practical importance for the design of WFSV. It is known that correlation between  
 280 heave and pitch motions is an important factor, and it has influence on the shape of bivariate contour lines.

281

282

## 283 6 Bivariate integral correction

284

285 Apart from the obvious prediction of bivariate contours with a given extreme return period, one could mention  
 286 another possible use, given high correlation between two studied univariate variables, namely vessel heave and pitch.  
 287 Indeed Fig. 8 exhibits high cross-correlation between two univariate responses with correlation coefficient  $R_{\text{corr}} =$   
 288 0.75.

289 The bivariate correction is based on the ACER methodology. The latter involves both the univariate ACER functions  
 290 as well as the bivariate ACER functions. The unique feature of the ACER functions is that they provide the  
 291 possibility to portray the exact extreme value distribution inherent in the data time series, both the univariate as well  
 292 as the bivariate, see [Gaidai et al. \(2019a\)](#); [Gaidai et al. \(2019b\)](#); [Xu et al \(2019b\)](#); [Karpa \(2015\)](#). For the sake of easy  
 293 reference, the bivariate ACER methodology has been briefly outlined in the Appendix.

294

295 This section presents a statistical bivariate integral correction that is based on the bivariate ACER method coupled  
 296 with the Gumbel logistic model, see [Karpa \(2015\)](#). Note that this correction is not limited to only extreme value  
 297 estimates, but it can be applied with appropriate bivariate models for any statistical values of interest, in order to  
 298 improve their accuracy based on synchronously measured longer, highly correlated data records.

299 To fix ideas, let  $Q(t)$  denote the hawser tension of interest, which has been recorded over a time interval of length  $\tilde{T}$ .  
 300 Assuming that  $\tilde{T}$  is insufficient for accurately predicting the desired univariate extreme tension level with a target  
 301 low probability of exceedance for a time period much longer than  $\tilde{T}$ , e.g. 3 hours during a design sea state. Typically,  
 302 an estimate of the tension level with a return period of 100 times longer may be desired as a design level. This would  
 303 be formulated as follows: Let  $Y = \max\{Q(t); (t_0, t_0 + 3 \text{ hours})\}$ , where  $t_0$  is a suitable reference time. The design  
 304 return period level  $\eta_*$  then satisfies the equation  $\text{Prob}\{Y > \eta_*\} = 10^{-2}$ .

305

306 Let  $X = \max\{P(t); (t_0, t_0 + 3 \text{ hours})\}$ , and let  $F_{XY}(\xi, \eta) = \text{Prob}(X \leq \xi, Y \leq \eta)$  be the joint bivariate cumulative  
 307 distribution function (CDF) of  $(X, Y)$ , and let  $F_X(\xi)$  and  $F_Y(\eta)$  be the corresponding univariate marginal CDFs for  
 308  $X$  and  $Y$ , respectively. As discussed in the previous section, in this paper it is assumed that the bivariate couple  
 309  $(P(t), Q(t))$  has been observed over a period of time  $t \in [0, \tilde{T}]$ , where the observation duration  $\tilde{T}$  is not long enough  
 310 for accurately predicting the univariate extreme response levels with a target low probability of interest. Now,  
 311 consider the case when a 'long' record of  $P(t)$  is available over a time  $t \in [0, T]$ , with  $T \gg \tilde{T}$ , with a corresponding  
 312 estimated CDF  $F_X^{\text{long}}(\xi)$  of the CDF  $F_X(\xi)$ , which has a probability density function (PDF)  $p_X = F_X'$ . Then for any  
 313  $Y$ -response level of interest  $\eta_*$ , with  $\Delta \rightarrow 0$ ,

314

315  $F_Y(\eta_*) = F_{XY}(\infty, \eta) = \int_0^{+\infty} \text{Prob}(Y \leq \eta_* | X = \xi) p_X(\xi) d\xi =$

316

317  $\int_0^{+\infty} \frac{\text{Prob}(Y \leq \eta_*, X \in [\xi, \xi + \Delta])}{\text{Prob}(X \in [\xi, \xi + \Delta])} p_X(\xi) d\xi = \int_0^{+\infty} F'_{XY,X}(\xi, \eta_*) d\xi,$  (A)

318

319 with  $F'_{XY,X}(\xi, \eta_*)$  denoting the derivative of  $F_{XY}(\xi, \eta_*)$  with respect to  $\xi$ .

320

321 The following copula model for the bivariate extreme value distribution is referred to as the Gumbel logistic model

322 Karpa (2015)

323

324  $F_{XY}(\xi, \eta) = \exp \left\{ - \left[ (-\ln F_X(\xi))^m + (-\ln F_Y(\eta))^m \right]^{1/m} \right\}$  (B)

325

326 This bivariate extreme value model has been verified to be a useful model for practical applications provided the

327 marginal extreme value distributions are estimated using the univariate ACER method instead of standard asymptotic

328 extreme value distributions, see Karpa (2015). If Eq. (4) is differentiated with respect to  $\xi$ , and substituted into the

329 integrand of Eq. (A), it is obtained that

330

331  $F'_{XY,X}(\xi, \eta_*) = F_{XY}(\xi, \eta_*) \left[ 1 + \left( \ln F_Y(\eta_*) / \ln F_X(\xi) \right)^m \right]^{\frac{1}{m}-1} \frac{d}{d\xi} \ln F_X(\xi)$  (C)

332

333 The numerical estimate  $\hat{F}_Y(\eta_*)$  of  $F_Y(\eta_*)$  based on the available time series of recorded data, is obtained by using the

334 following expression,

335

336  $\hat{F}_Y(\eta_*) = \int_0^{+\infty} \hat{F}_{XY}(\xi, \eta_*) \left[ 1 + \left( \ln \hat{F}_Y(\eta_*) / \ln \hat{F}_X^{long}(\xi) \right)^m \right]^{\frac{1}{m}-1} \frac{d}{d\xi} \ln \hat{F}_X^{long}(\xi) d\xi$  (D)

337

338

339 By applying suggested bivariate correction, the accuracy of extreme response prediction was greatly improved, as

340 discussed here under.

341 The ‘‘long’’ 3-hour record was chosen as the reference; then the relative ratio of the prediction based on the ‘‘short’’

342 0.3 hours observation  $x_{short}$  has been divided by the prediction  $x_{long}$  based on longer 3 hours observation. The results

343 are summarized in the following table.

344

345 **Table 3 Bivariate correction results (long term analysis).**

	heave	pitch
$x_{short}/x_{long}$	0.81	0.80
$x_{corrected}/x_{long}$	1.05	1.06

346

347

348 Table 3 shows that there is a practical advantage in applying the bivariate correction introduced in this paper, as it

349 brings shorter prediction quite close to the longer prediction, thus greatly improving accuracy of the extreme value

350 prediction. Note that as mentioned above, a high correlation between two processes is the key requirement for the

351 described correction in order to achieve improved prediction accuracy.

## 352 7 Conclusions

353

354

355 This paper presents a study of the combined motion statistics of a wind farm support vessel, during fender docking

356 against a wind tower in actual random seas.

357

358 The state-of-art bivariate ACER method has been described and applied to study the vessel coupled motions,

359 specifically heave and pitch, simulated synchronously in time. Low probabilities (high quantiles) contours for the

360 bivariate extreme value distribution have been obtained by adopting bivariate copula models.

361 When it comes to safety and reliability of WFSV operations, the multivariate analysis is a more appropriate approach,

362 than the typically used univariate approach. The presented methodology has the following important advantages:

- 363
- 364
- 365 ■ Unlike IRORM or SORM, the ACER2D do not simplify non-linearities, inherent in the model.
  - 366 ■ Any kind of coupled data can be analysed: either measured or obtained by Monte Carlo simulation.
  - 367 ■ Non-stationary data can be analysed.

- 367 ■ Unlike asymptotically based methods (Gumbel, Pareto, Weibull, Peak over threshold etc.) the presented method
- 368 can accommodate pre-asymptotic behaviour, which means that the data set can be analysed more accurately and
- 369 efficiently.
- 370 ■ A bivariate correction can be done based on the bivariate statistical analysis for two highly correlated variables.
- 371 The latter can have practical applications at the design stage.

372  
373 It should be acknowledged that in order to deliver solid and reliable design analysis for industrial operational

374 purposes, one has to conduct physical tests, and validate numerical simulation results versus experimental. This

375 paper however has been focused on developing an efficient statistical methodology, rather than on hydrodynamic and

376 structural interaction aspects.

## 377 378 379 **8 Appendix: bivariate ACER2D method (in brief)**

380  
381 This paper analyzes bivariate random process  $Z(t) = (X(t), Y(t))$ , consisting of two scalar component processes

382  $X(t), Y(t)$ , measured or simulated synchronously, over certain period of time  $(0, T)$ . It is assumed that samples

383  $(X_1, Y_1), \dots, (X_N, Y_N)$  are recorded at  $N$  time equidistant discrete instants  $t_1, \dots, t_N$  within  $(0, T)$ . Note that the latter

384 assumption of equidistant time sampling serves convenience purpose and does not limit the described methodology.

385 This Appendix studies the joint cumulative distribution function (CDF)  $P(\xi, \eta) := \text{Prob}(\hat{X}_N \leq \xi, \hat{Y}_N \leq \eta)$  of the

386 maxima vector  $(\hat{X}_N, \hat{Y}_N)$ , with  $\hat{X}_N = \max\{X_j; j = 1, \dots, N\}$ , and  $\hat{Y}_N = \max\{Y_j; j = 1, \dots, N\}$ . In this paper  $\xi$  and  $\eta$

387 are the heave and pitch angle correspondingly, recorded synchronously at the vessel COG.

388 Following non-exceedance event is defined  $\mathcal{C}_{kj}(\xi, \eta) := \{X_{j-1} \leq \xi, Y_{j-1} \leq \eta, \dots, X_{j-k+1} \leq \xi, Y_{j-k+1} \leq \eta\}$  for

389  $1 \leq k \leq j \leq N + 1$ . According to the definition of CDF  $P(\xi, \eta)$

$$\begin{aligned}
 P(\xi, \eta) &= \text{Prob}(\mathcal{C}_{N+1, N+1}(\xi, \eta)) \\
 &= \text{Prob}(X_N \leq \xi, Y_N \leq \eta \mid \mathcal{C}_{NN}(\xi, \eta)) \cdot \text{Prob}(\mathcal{C}_{NN}(\xi, \eta)) \\
 &= \prod_{j=2}^N \text{Prob}(X_j \leq \xi, Y_j \leq \eta \mid \mathcal{C}_{jj}(\xi, \eta)) \cdot \text{Prob}(\mathcal{C}_{22}(\xi, \eta))
 \end{aligned} \tag{1}$$

392  
393 The target CDF  $P(\xi, \eta)$  can be represented as in Naess and Karpa (2015); Karpa and Naess (2015); Gaidai et al.

394 (2016)

$$P(\xi, \eta) \approx \exp\left\{-\sum_{j=k}^N (\alpha_{kj}(\xi; \eta) + \beta_{kj}(\eta; \xi) - \gamma_{kj}(\xi, \eta))\right\}; \xi, \eta \rightarrow \infty \tag{2}$$

395  
396 for sufficiently the large conditioning order parameter  $k$  with  $\alpha_{kj}(\xi; \eta) := \text{Prob}(X_j > \xi \mid \mathcal{C}_{kj}(\xi, \eta)), \beta_{kj}(\eta; \xi) :=$

397  $\text{Prob}(Y_j > \eta \mid \mathcal{C}_{kj}(\xi, \eta)), \gamma_{kj}(\xi, \eta) := \text{Prob}(X_j > \xi, Y_j > \eta \mid \mathcal{C}_{kj}(\xi, \eta))$ .

398 From Eq. (2) shows that accurate estimate of the bivariate CDF  $P(\xi, \eta)$  requires accurate estimate of functions

399  $\{(\alpha_{kj}(\xi; \eta) + \beta_{kj}(\eta; \xi) - \gamma_{kj}(\xi, \eta))\}_{j=k}^N$ . Let one introduce  $k$ -th order bivariate average conditional exceedance rate

400 (ACER2D) functions

$$\mathcal{E}_k(\xi, \eta) = \frac{1}{N - k + 1} \sum_{j=k}^N (\alpha_{kj}(\xi; \eta) + \beta_{kj}(\eta; \xi) - \gamma_{kj}(\xi, \eta)), \quad k = 1, 2, \dots \tag{3}$$

402 Then, when  $N \gg k$ , one has

$$P(\xi, \eta) \approx \exp\{- (N - k + 1) \mathcal{E}_k(\xi, \eta)\}; \xi, \eta \rightarrow \infty \tag{4}$$

404 Estimation of bivariate ACER2D functions  $\mathcal{E}_k(\xi, \eta)$  for the simulated/observed stationary/non-stationary time series

405 constitutes counting of exceedance events. For the detailed estimation of the bivariate ACER2D functions, see Naess

406 and Karpa (2015); Karpa and Naess (2015); Gaidai et al. (2016).

407 In the above, no assumption about stationarity has been made. Hence, it also applies to long term time series.

408 However, in this case it is often expedient to reformulate the expressions. Given the scatter diagram consisting of

409  $m = 1, \dots, M$  sea states (long term statistics), each sea state having an individual probability  $p_m$ , with the obvious

410 equality  $\sum_{m=1}^M p_m = 1$ . Then it is natural to rewrite the ACER2D function definition as follows  $\mathcal{E}_k(\xi, \eta) =$

411  $\sum_{m=1}^M \mathcal{E}_k(\xi, \eta, m) p_m$ , with  $\mathcal{E}_k(\xi, \eta, m)$  being the ACER2D function estimated for an individual  $m$ -th stationary sea

412 state. This provides a practical expression for the long term ACER2D function.

413

414 For any couple of random variables  $(X, Y)$  with marginal CDFs  $F_x(\xi)$  and  $G_y(\eta)$ , the joint CDF  $H_{xy}(\xi, \eta) =$   
 415  $\text{Prob}(X \leq \xi, Y \leq \eta)$  can be represented by the bivariate copula  $C(u, v)$  as  $H_{xy}(\xi, \eta) = C(F_x(\xi), G_y(\eta))$ , cf. e.g.  
 416 Balakrishnan and Lai (2009); Sklar (1959); Nelsen (2006); Yue et al. (1999); Yue and Wang (2004); Hougaard  
 417 (1986); Tiago de Oliveira (1984). The latter is a general theoretical result that holds for any bivariate extreme value  
 418 distribution.

419 In this paper two types of Pickands dependence function have been applied: The optimized Gumbel logistic (GL)  
 420 Gumbel (1960a); Gumbel (1960b); Gumbel (1961) and the optimized Asymmetric logistic (AL), Coles and Tawn  
 421 (1991); Coles and Tawn (1994); Coles (2001); Tawn (1988); Balakrishnan and Lai (2009); Sklar (1959); Nelsen  
 422 (2006); Pickands (1981); Gudendorf and Segers (2010).

423  
 424 It is now assumed that the marginal extreme value distributions (EVD) are represented by the univariate ACER  
 425 functions, see Naess and Karpa (2015); Karpa and Naess (2015); Gaidai et al. (2016)  
 426

$$\begin{aligned} F_x(\xi) &\approx \exp\{-(N - k + 1)\varepsilon_k^x(\xi)\}, & \xi \geq \xi_1 \\ G_y(\eta) &\approx \exp\{-(N - k + 1)\varepsilon_k^y(\eta)\}, & \eta \geq \eta_1 \end{aligned} \quad (5)$$

427  
 428 with  $\varepsilon_k^x(\xi) = q_k^x \exp\{-a_k^x(\xi - b_k^x)^{c_k^x}\}$  and similar definition for  $\varepsilon_k^y(\eta)$ .  
 429

430 The detailed optimization procedure for estimation of parameters  $a_k^x, b_k^x, c_k^x, q_k^x, a_k^y, b_k^y, c_k^y, q_k^y$  is outlined in Naess  
 431 and Karpa (2015); Karpa and Naess (2015); Gaidai et al. (2016).  
 432

433 The choice of copula model is now determined by the accuracy obtained by fitting  $H_{xy}(\xi, \eta)$  to the empirical  $P(\xi, \eta)$ .  
 434

## 435 References

436 Bondarenko, O. (2015), "The Statistical Analysis of Principal Particulars of Wind Farm Support Vessels",  
 437 Second Design & Operation of offshore Wind Farm Vessels conference, Royal Institution of Naval Architects,  
 438 London.

439 Wu, M. K. (2014), "Numerical analysis of docking operation between service vessels and offshore wind  
 440 turbines", *Ocean Engineering*, Vol. 91, pp. 379-388.  
 441

442 Zhang, P., Dai, L., Liu, Y., (2014a), "Modelling Operation of Service Vessels in Offshore Wind Farms Using  
 443 Stochastic Activity Networks", *Ship Technology Research*, Vol. 61(1), 48-58, DOI: 10.1179/str.2014.61.1.004

444 Naess, A, and Moan, T. (2013), "Stochastic dynamics of marine structures", Cambridge University Press.

445  
 446 Price, W.G., and Bishop, R.E.D. (1974), "Probabilistic theory of ship dynamics", Chapman and Hall, London.  
 447

448 Sandvik, P.C. (2010), "Docking application and significant waves - simulation of dynamic behavior during  
 449 docking for personnel transfer", Kockum Workshop on service vessels for personnel transportation to offshore  
 450 wind turbines, Karlskrona, Sweden.  
 451

452 Zhang, L, and Zhu, R, and Niu, F. (2014b), "Offshore wind power operational ship design analysis", *Journal*  
 453 *International offshore wind*, Vol. 6, pp. 62-66.  
 454

455 Phillips, S, and Shin, I, and Armstrong, C. (2014), "Performance evaluation of wind-farm support vessels",  
 456 Design & Operation of Wind Farm Support Vessels, London, UK.

457 Phillips, S, and Shin, I, and Armstrong, C. (2015), "Crew transfer vessel performance evaluation", Design &  
 458 Operation of Wind Farm Support Vessels, London, UK.

459 Naess, A, and Karpa, O. (2015), "Statistics of bivariate extreme wind speeds by the ACER2D method", *J. Wind*  
 460 *Eng. and Ind. Aerodyn.* Vol 139, pp.82-88.  
 461

462 Karpa, O., and Naess, A. (2015), "Statistics of Extreme Wind Speeds and Wave Heights by the Bivariate  
 463 ACER2D Method", *J. Offshore Mech. and Arctic Eng.*, 137(2).  
 464

- 465 Gaidai, O, and Storhaug, G, and Naess, A. (2016), "Extreme large cargo ship panel stresses by bivariate  
466 ACER2D method", Vol. 127, pp. 368-386.
- 467  
468 Heffernan, J.E., and Tawn, J.A. (2004), "A Conditional Approach for Multivariate Extreme Values", Journal of  
469 the Royal Statistic Society: Series B, 66(3), pp. 497—546.
- 470  
471 Ewans, K. (2014), "Evaluating Environmental Joint Extremes for the Offshore Industry Using the Conditional  
472 Extremes Model", Journal of Marine Systems, 130, pp. 124-130.
- 473  
474 Zhao, Y.G., and Ono, T. (1999), "A general procedure for first/second order reliability method  
475 (FORM/SORM)", Structural safety 21(2), pp. 95-112.
- 476  
477 <http://www.globalwavestatisticsonline.com/>
- 478  
479 DNV-RP-H103, (2011), "Modelling and analysis of marine operations".
- 480  
481 DNV-RP-C205, (2010), "Environmental conditions and environmental loads".
- 482  
483 Wang, Z., and Wu, K., and Zhou, L., and Wu, L., (2012), "Wave characteristics and extreme parameters in the  
484 Bohai sea", China Ocean Engineering, Vol. 26(2), pp. 341- 350.
- 485  
486 Gumbel, E. J. (1960a), "Multivariate Extremal Distributions", Bull. l'Institut Int. Stat., 37(2), pp. 471–475.
- 487  
488 Gumbel, E. J. (1960b), "Bivariate Exponential Distributions", J. Am. Stat. Assoc., 55(292), pp. 698–707.
- 489  
490 Gumbel, E. J. (1961), "Bivariate Logistic Distributions", J. Am. Stat. Assoc., 56(294), pp. 335–349.
- 491  
492 Gumbel, E. J., and Mustafi, C. K. (1967), "Some Analytical Properties of Bivariate Extremal Distributions", J.  
493 Am. Stat. Assoc., 62(318), pp. 569–588.
- 494  
495 Coles, S. G., and Tawn, J. A. (1991), "Modeling Extreme Multivariate Events", J. R. Stat. Soc. B, 53(2), pp.  
496 377–392.
- 497  
498 Coles, S. G., and Tawn, J. A. (1994), "Statistical Methods for Multivariate Extremes: An Application to  
499 Structural Design", J. R. Stat. Soc. C, 43(1), pp. 1–48.
- 500  
501 Coles, S. (2001), "An Introduction to Statistical Modeling of Extreme Values", Springer Series in Statistics,  
502 Springer-Verlag, London, UK.
- 503  
504 Tawn, J. A. (1988), "Bivariate Extreme Value Theory: Models and Estimation", Biometrika, 75(3), pp. 397–415.
- 505  
506 Balakrishnan, N., and Lai, C.D. (2009), "Continuous Bivariate Distributions", Springer Science Business Media,  
507 New York.
- 508  
509 Sklar, M. (1959), "Fonctions de repartition dimensions et leurs marges", Publications of the Institute of Statistics  
510 of the University of Paris, Paris, France, Vol. 8, pp. 229–231.
- 511  
512 Nelsen, R. B. (2006), "An Introduction to Copulas (Springer Series in Statistics) ", Springer Science Business  
513 Media, New York.
- 514  
515 Pickands, J. (1981), "Multivariate Extreme Value Distributions", Proceedings of the 43rd Session of the  
516 International Statistical Institute, Bulletin of the International Statistical Institute, Vol. 49, pp. 859–878.
- 517  
518 Gudendorf, G., and Segers, J. (2010), "Extreme-Value Copulas", Copula Theory and Its Applications, Springer-  
519 Verlag, Berlin, Germany, Chap. 6.
- 520  
521 Yue, S., and Ouarda, T., and Bobee, B., and Legendre, P., and Bruneau, P. (1999), "The Gumbel Mixed Model  
522 for Flood Frequency Analysis", J. Hydrol., 226(1–2), pp. 88–100.
- 523  
524 Yue, S., and Wang, C. Y. (2004), "A Comparison of Two Bivariate Extreme Value Distributions", Stochastic

523 Environ. Res. Risk Assessment, 18(2), pp. 61–66.  
524  
525 Hougaard, P. (1986), "A Class of Multivariate Failure Time Distributions", *Biometrika*, 73(3), pp. 671–678.  
526  
527 Tiago de Oliveira, J. (1984), "Bivariate Models for Extremes; Statistical Decision", *Statistical Extremes and*  
528 *Applications*, Springer, New York, pp. 131–153.  
529  
530 AQWA users manual, (2013), Release 15.0, ANSYS, Inc.  
  
531  
532 AQWA theory manual, (2013), Release 15.0, ANSYS, Inc.  
  
533  
534 Karpa, O., 2015, "Development of bivariate extreme value distributions for applications in marine technology",  
535 *Marine Technology*, NTNU, Trondheim.  
  
536  
537 Lv, X., Yuan, D., Ma, X., Tao, J., 2014. Wave characteristics analysis in Bohai Sea based on ECMWF wind  
538 field. *Ocean Engineering*, Vol 91, pp. 159-171.  
  
539  
540 Zhang, J., Gaidai, O., Gao, J., (2018). "Bivariate extreme value statistics of offshore jacket support stresses in  
541 Bohai bay", *J. Offshore Mech. Arctic Eng.* 140 (4), 041305. <https://doi.org/10.1115/1.4039564>.  
  
542  
543 Reed, A.M. and Beck, R.F. (2016). "Advances in the Predictive Capability for Ship Dynamics in Extreme  
544 Waves", *SNAME Transactions*, Vol. 124.  
  
544  
545 Xu, X., Sahoo, P., Evans, J., and Tao, Y., (2019a), "Hydrodynamic performances of FPSO and shuttle tanker  
546 during side-by-side offloading operation, ships and offshore structures",  
<https://doi.org/10.1080/17445302.2019.1580845>.  
  
547  
548 Xu, X., Gaidai, O., Naess, A., Sahoo, P., (2019b), "Improving the prediction of extreme FPSO hawser tension,  
549 using another highly correlated hawser tension with a longer time record", *Applied Ocean Research*, Vol. 88,  
pp. 89–98.  
  
550 DNV-RP-H103, (2011). *Modelling and analysis of marine operations*.  
551 Naess, A., Gaidai, O., and Haver, S., 2007. "Estimating extreme response of drag dominated offshore structures  
552 from simulated time series of structural response". *Ocean Engineering*, Vol 34, pp. 2188-2197.  
553 Gaidai, O., Naess, A., Karpa, O., Xu, X., Cheng, Y., Ye, R. (2019a), "Improving extreme wind speed prediction  
554 for North Sea offshore oil and gas fields", *Applied Ocean Research*, Vol 88, pp. 63–70.  
555 Gaidai, O., Naess, A., Xu, X, Cheng, Y, (2019). "Improving extreme wind speed prediction based on a short data  
556 sample, using a highly correlated long data sample", *Journal of Wind Engineering & Industrial Aerodynamics*,  
557 Vol 188, pp. 102–109.  
558