| 1  | Modelling creep in clay using the framework of hyper-viscoplasticity   |  |  |  |  |
|----|--|--|--|--|--|
| 2  |  |  |  |  |  |
| 3  | GUSTAV GRIMSTAD*1, DAVOOD DADRASAJIRLOU* and SEYED ALI   |  |  |  |  |
| 4  | GHOREISHIAN AMIRI*   |  |  |  |  |
| 5  | *PoreLab, Department of Civil and Environmental Engineering, Norwegian University of                         |  |  |  |  |
| 6  | Science and Technology (NTNU), Trondheim, Norway   |  |  |  |  |
| 7  |  |  |  |  |  |
| 8  | <sup>1</sup> corresponding author, e-mail: <u>gustav.grimstad@ntnu.no</u> , address: Department of Civil and |  |  |  |  |
| 9  | Environmental Engineering, Høgskoleringen 7A, 7491 Trondheim, Norway   |  |  |  |  |
| 10 |  |  |  |  |  |
| 11 | Abstract: This paper addresses the derivation of creep models using the framework of hyper-                  |  |  |  |  |
| 12 | viscoplasticity. It demonstrates that the formulations widely used already can easily be                     |  |  |  |  |
| 13 | obtained using the hyper-viscoplastic formalism. This means that existing formulations (i.e.                 |  |  |  |  |
| 14 | of the flow potential) are thermodynamically sound. The key assumptions are that the free                    |  |  |  |  |
| 15 | energy is only a function of elastic strains and that there is no dissipation under pure                     |  |  |  |  |
| 16 | volumetric swelling (tension). The presented derivations, using the framework of hyper-                      |  |  |  |  |
| 17 | viscoplasticity, allows for further model development along the same lines, as presented here,               |  |  |  |  |
| 18 | with only minor modifications.   |  |  |  |  |
| 19 |  |  |  |  |  |
| 20 | KEYWORDS: time dependence; creep; compressibility; clays; constitutive relations;                            |  |  |  |  |
| 21 | plasticity   |  |  |  |  |
| 22 |  |  |  |  |  |

### 23 INTRODUCTION

Only a few attempts have been made to formulate a hyper-viscoplastic model for creep and 24 25 rate dependence of soft clays that comply with the critical state soil mechanics concept. There 26 is still a need for showing the derivations of such a model in a complete formalistic manner. 27 Attempts has been made, e.g. Aung et al. (2019), but still some clarifications are necessary to 28 demonstrate a thermodynamic sound formulation. Therefore, this paper demonstrates how to 29 establish the well-known empirically based formulation for creep in clay from a 30 thermodynamic perspective. The result is in strong resemblance to the creep formulation 31 widely used in geotechnics already. As found in Šuklje (1963), Janbu (1969) and others, for 32 1D case, and extended to full stress space in e.g. Stolle et al. (1999), with the effect of fabric 33 by Leoni et al. (2008). The presented derivation from the thermodynamic framework gives 34 the same correction to the Stolle et al. (1999) formulation as suggested by Grimstad et al. (2008) to properly model the "dry side", as further discussed in Grimstad et al. (2010). The 35 36 notation used, follows the book of Houlsby and Puzrin (2006), with concepts/terminology discussed in e.g. Collins and Kelly (2002), Collins and Houlsby (1997), Darabi et al. (2018) 37 38 and Osman et al. (2020). Small strains are assumed, so additive decomposition of elastic and 39 viscoplastic strains holds. Cauchy stresses are hence then also used. Triaxial stress (p-q)40 space is utilized to simplify the derivation, but extension to full stress space is 41 straightforward. This note makes use of the normal geotechnical sign convention, i.e. 42 compression positive. The principles of hyper-viscoplasticity are briefly presented in the 43 appendix.

44

### 45 DERIVATION OF THE FLOW POTENTIAL

46 The starting point of developing models, using the thermodynamic framework, is to establish 47 the free energy function and the force potential. In terms of Helmholtz free energy, f, it can 48 take the form of eq. (1), where the free energy is implicitly a function of the elastic strain 49 only.

50 
$$f = \kappa \cdot p_{ref} \cdot \exp\left(\frac{\varepsilon_{\nu} - \varepsilon_{\nu}^{\nu p}}{\kappa}\right) + \frac{3G}{2} \cdot \left(\varepsilon_{q} - \varepsilon_{q}^{\nu p}\right)^{2}$$
(1)

51 Where *G* is the shear stiffness,  $\kappa$  is elastic compressibility parameter (the bulk stiffness 52 increases linearly with mean effective stress),  $p_{ref}$  is an arbitrary reference pressure,  $\varepsilon_v$  is the 53 volumetric strain and  $\varepsilon_q$  is the deviatoric strain, energy conjugates to the mean effective 54 stress, *p*, and deviatoric stress, *q*, respectively. Elastic strains are defined as total strains 55 minus viscoplastic strains,  $\varepsilon_v - \varepsilon_v^{vp}$  and  $\varepsilon_q - \varepsilon_q^{vp}$ . Alternatively, the free energy, eq. (1), can be 56 expressed in terms of Gibbs free energy, *g*:

57 
$$g = -\kappa \cdot p \cdot \left( \ln \left( \frac{p}{p_{ref}} \right) - 1 \right) - \frac{q^2}{6G} - \left( p \cdot \varepsilon_v^{vp} + q \cdot \varepsilon_q^{vp} \right)$$
(2)

This form of free energy results in true stress equals to dissipative stress (see appendix), thus,
one can further concentrate only on the force potential.

60 Consider the following force potential, *z*:

61 
$$z = \frac{p_0}{2^n} \cdot \frac{r^{1-n}}{n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2} + \dot{\varepsilon}_v^{vp}\right)^n$$
(3)

Where  $p_0$  is a state variable equivalent to the isotropic "pre-consolidation" stress, r is a reference rate, n is a number slightly larger than 1 but significantly less than 2. n = 2 would mean linear increase with strain rate. From experiments it is well documented that this has a logarithmic nature, see e.g. the early work of Buisman (1936), Šuklje (1963) or Bjerrum (1967): this implies a n between 1 and 2. M is the critical state line in p-q space. This force potential gives expectation of a behavior that scales the behavior of the Modified Cam-Clay model (MCCM) (Roscoe and Burland, 1968). In fact, with n = 1, the dissipation function of the MCCM is retrieved from this force potential in a form as discussed in e.g. Collins and Houlsby (1997), with the "shift" included in the dissipation function.

71 The dissipation, *d*, equals, after differentiation:

$$72 \qquad d = \frac{\partial z}{\partial \dot{\varepsilon}_{v}^{vp}} \cdot \dot{\varepsilon}_{v}^{vp} + \frac{\partial z}{\partial \dot{\varepsilon}_{q}^{vp}} \cdot \dot{\varepsilon}_{q}^{vp} = \frac{p_{0}}{2^{n}} \cdot r^{1-n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_{v}^{vp}\right)^{2} + M^{2} \cdot \left(\dot{\varepsilon}_{q}^{vp}\right)^{2}} + \dot{\varepsilon}_{v}^{vp}\right)^{n} \ge 0$$

$$\tag{4}$$

See that  $d \ge 0$  holds for any strain rate. For pure volumetric unloading, it results in d = 0. It means under pure volumetric swelling, there is no dissipation, as all applied energy is spent in volume increase. One will see later that this happens only in the Origin of stress space, which is a realistic behavior in many cases (i.e. liquefaction). However, when using a free energy function as the one in eq. (1), such a state is actually impossible, as it will require infinite negative elastic volumetric strain to reach zero mean effective stress.

79 The flow potential, *w*, is found from the difference between dissipation and force potential:

80 
$$w = d - z = \frac{p_0}{2^n} \cdot \frac{n-1}{n} \cdot r^{1-n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2} + \dot{\varepsilon}_v^{vp}\right)^n$$
 (5)

81 (note again that for n = 1 the dissipation is linear in strain rate, and w will then define the 82 yield surface (y) in dissipative stress space ( $\chi$ ); generally  $y(\chi_{ij}, \sigma_{ij}, p_0)$  as w = y = 0). Which, in 83 true stress space, will give the plastic potential function and yield surface. In this case, it will 84 result in an associated flow rule as  $y = y(\chi_{ij}, p_0)$ . However, if e.g.  $M = M(\sigma_{ij})$ , a non-associated 85 flow rule will be predicted.

86 The dissipative stresses are derived from the force potential as:

$$87 \qquad \begin{bmatrix} \chi_p \\ \chi_q \end{bmatrix} = \begin{bmatrix} \frac{\partial z}{\partial \dot{\varepsilon}_v^{vp}} \\ \frac{\partial z}{\partial \dot{\varepsilon}_q^{vp}} \end{bmatrix} = \frac{\frac{p_0}{2^n} \cdot r^{1-n} \cdot \left( \sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2} + \dot{\varepsilon}_v^{vp} \right)^n}{\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2}} \cdot \left[ \frac{M^2 \cdot \dot{\varepsilon}_q^{vp}}{\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2}} \right] \tag{6}$$

88 Rearranging the equation results in:

$$89 \qquad \dot{\varepsilon}_{q}^{vp} = \frac{2 \cdot \frac{\chi_{q}}{\chi_{p}}}{M^{2} - \left(\frac{\chi_{q}}{\chi_{p}}\right)^{2}} \cdot \dot{\varepsilon}_{v}^{vp} \tag{7}$$

20 Zero shift stress (i.e.  $\chi_{ij} = \sigma_{ij}$ ) and assumption of maximum dissipation rate (i.e.  $\chi_{ij} = \chi_{ij}$  the 21 Ziegler's orthogonality assumption (Ziegler, 1983), see the appendix), results in the well-22 known MCCM flow rule in the true stress space:

93 
$$\dot{\varepsilon}_{q}^{vp} = \frac{2 \cdot \frac{q}{p}}{M^{2} - \left(\frac{q}{p}\right)^{2}} \cdot \dot{\varepsilon}_{v}^{vp}$$
(8)

Rearranging and eliminating the viscoplastic strain rates give the flow potential as a functionof dissipative stresses:

96 
$$w = p_0 \cdot r \cdot \frac{n-1}{n} \cdot \left(\frac{p_{eq}}{p_0}\right)^{\frac{n}{n-1}}$$
(9)

97 Where, one may identify the known equivalent stress measure,  $p_{eq}$  as:

98 
$$p_{eq} = p + \frac{1}{M^2} \cdot \frac{q^2}{p}$$
 (10)

99 Note that eq. (9) results in a family of similar ellipses with a "similarity center" at the origin100 of the true stress space.

101

# 102 RESULTS AND DISCUSSION

103 The derived flow potential, eq. (9), can be used directly for modelling creep in clay.

104 However, the material parameters *r* and *n* does not give direct physical meaning. Therefore, it

105 is more convenient to change them to engineering ones. By comparing eq. (9) with the

106 formulation for the plastic multiplier found in e.g. Grimstad et al. (2010) (after integration),

107 one gets:

108 
$$w = p_0 \cdot \frac{\dot{\lambda}_{ref}}{\frac{\lambda - \kappa}{\mu} + 1} \cdot \left(\frac{p_{eq}}{p_0}\right)^{\frac{\lambda - \kappa}{\mu} + 1} = p_0 \cdot r \cdot \frac{n - 1}{n} \cdot \left(\frac{p_{eq}}{p_0}\right)^{\frac{n}{n-1}}$$
(11)

109 As a result, *n* and *r* relates to "classical" parameters through:

110 
$$n = \frac{\lambda - \kappa + \mu}{\lambda - \kappa} \text{ and } \dot{\lambda}_{ref} = r$$
 (12)

111 For typical values of  $\lambda - \kappa$  (= 0.09) and  $\mu$  (= 0.0036) for soft clays (i.e. a creep number

112  $[(\lambda - \kappa)/\mu]$  of 25), one observes that the number *n* is 1.04. Note that all these parameters are 113 found from conventional laboratory tests.

114 Further, see that the over consolidation ratio, *OCR*, is identified as:

115 
$$OCR = \frac{p_0}{p_{eq}}$$
(13)

116 Also, in practice, it is not convenient to use a reference rate as input parameter. Therefore,

- 117 Grimstad et al. (2010), already defined this in terms of more conventional parameters
- 118 through:

119 
$$\dot{\lambda}_{ref} = \frac{\mu}{\tau} \cdot \frac{M^2}{M^2 - \eta_{K0NC}^2}$$
 (14)

120 Where,  $\tau$  is the reference time for which  $p_0$  (or *OCR*) is determined, typically 1 day for 121 incrementally loaded oedometer tests. The last term is there to generalize the oedometer 122 condition to general condition, where  $\eta_{KONC}$  is the stress ratio, q/p, under 1D normal 123 compression. This derivation shows that the creep formulation used in e.g. Grimstad et al. (2017) (with  $OCR_{max} \rightarrow \infty$ ) can be exactly derived in the framework of hyper-viscoplasticity. 124 125 With a modified force potential, e.g. including the effect of fabric and Lode angle 126 dependency, a more advance model could be retrieved following the same steps as above. 127 Also, if a linear term is added to the force potential, it is quite straightforward to include a 128 type of  $OCR_{max}$  parameter, ending up with a formulation with some similar characteristics as 129 the one suggested in Grimstad and Degago (2010) and Grimstad et al. (2017), with OCR<sub>max</sub> 130 representing an inner limit surface corresponding to zero viscoplastic strain rate.

131 To complete the scheme, the viscoplastic strain rates are calculated from the flow potential,132 giving the following viscoplastic strain rates:

133 
$$\begin{bmatrix} \dot{\varepsilon}_{v}^{vp} \\ \dot{\varepsilon}_{q}^{vp} \end{bmatrix} = \begin{bmatrix} \frac{\partial w}{\partial \chi_{p}} \\ \frac{\partial w}{\partial \chi_{q}} \end{bmatrix} = \dot{\lambda}_{ref} \cdot \left( \frac{p_{eq}}{p_{0}} \right)^{\frac{\lambda - \kappa}{\mu}} \cdot \left[ \frac{\frac{\partial p_{eq}}{\partial p}}{\frac{\partial p_{eq}}{\partial q}} \right]$$
(15)

134 The hardening rule of  $p_0$ , eq. (16), takes the same form as for MCCM, where strains are 135 considered rather than specific volume.

136 
$$\dot{p}_0 = \frac{p_0}{\lambda - \kappa} \cdot \dot{\varepsilon}_v^{vp} \tag{16}$$

Finally, it is interesting to note that when there is a difference between the dissipative stress and the true stress (coming from a different choice of free energy function), it will result in a creep formulation where the "similarity center" is not in the origin of p-q space. As an 140 example, eq. (17) gives the Gibbs free energy for a case where the "similarity center" will be141 in the center of the Cam-Clay ellipse.

$$142 \qquad g = -\kappa \cdot p \cdot \left( \ln \left( \frac{p}{p_{ref}} \right) - 1 \right) - \frac{q^2}{6G} - \left( p \cdot \varepsilon_v^{vp} + q \cdot \varepsilon_q^{vp} \right) + \left( \lambda - \kappa \right) \cdot \frac{p_{0,ref}}{2} \cdot e^{\frac{\varepsilon_v^{vp}}{\lambda - \kappa}}$$
(17)

143 The generalized stresses are found by differentiation. And when combined with the integral 144 of eq. (16) from  $p_{0,ref}$  to  $p_0$ , one obtains:

145  

$$\overline{\chi}_{p} = -\frac{\partial g}{\partial \varepsilon_{v}^{vp}} = p - \frac{p_{0,ref}}{2} \cdot e^{\frac{\varepsilon_{v}^{p}}{\lambda - \kappa}} = p - \frac{p_{0}}{2}$$

$$\overline{\chi}_{q} = -\frac{\partial g}{\partial \varepsilon_{q}^{vp}} = q$$
(18)

146 Here one identifies the "shift stress"  $p - \chi_p$  as  $p_0/2$ . The force potential is now:

147 
$$z = \frac{p_0}{2^n} \cdot \frac{r^{1-n}}{n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2}\right)^n$$
(19)

148 From this, the calculation of differentials and the transformation to a flow potential as a

149 function of an equivalent stress measure is done once more, eq. (20) to (25):

150 
$$d = \frac{\partial z}{\partial \dot{\varepsilon}_{v}^{vp}} \cdot \dot{\varepsilon}_{v}^{vp} + \frac{\partial z}{\partial \dot{\varepsilon}_{q}^{vp}} \cdot \dot{\varepsilon}_{q}^{vp} = \frac{p_{0}}{2^{n}} \cdot r^{1-n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_{v}^{vp}\right)^{2} + M^{2} \cdot \left(\dot{\varepsilon}_{q}^{vp}\right)^{2}}\right)^{n} \ge 0$$
(20)

151 
$$w = d - z = \frac{p_0}{2^n} \cdot \frac{n-1}{n} \cdot r^{1-n} \cdot \left(\sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2}\right)^n$$
 (21)

152 
$$\begin{bmatrix} \chi_p \\ \chi_q \end{bmatrix} = \begin{bmatrix} \frac{\partial z}{\partial \dot{\varepsilon}_v^{vp}} \\ \frac{\partial z}{\partial \dot{\varepsilon}_q^{vp}} \end{bmatrix} = \frac{p_0}{2^n} \cdot r^{1-n} \cdot \left( \sqrt{\left(\dot{\varepsilon}_v^{vp}\right)^2 + M^2 \cdot \left(\dot{\varepsilon}_q^{vp}\right)^2} \right)^{n-2} \cdot \begin{bmatrix} \dot{\varepsilon}_v^{vp} \\ M^2 \cdot \dot{\varepsilon}_q^{vp} \end{bmatrix}$$
(22)

153 Flow rule:

154 
$$\dot{\varepsilon}_{q}^{vp} = \frac{1}{M^2} \cdot \frac{\chi_q}{\chi_p} \cdot \dot{\varepsilon}_{v}^{vp}$$
(23)

### 155 Using eq. (22) to transform eq. (21) gives:

156 
$$w = p_0 \cdot r \cdot \frac{n-1}{n} \cdot \left(\frac{2 \cdot p_{eq}^*}{p_0}\right)^{\frac{n}{n-1}}$$
(24)

157 Where:

158 
$$p_{eq}^* = \sqrt{\chi_p^2 + \frac{\chi_q^2}{M^2}}$$
 (25)

159 Which defines a family of ellipses with a similarity center at  $p_0/2$  in p-q stress space, as  $\chi_{ij} = \overline{\chi_{ij}}$  (see Figure 1, Alt. 2). Note that this type has similar characteristics as the flow 160 potential of Apriadi et al. (2013). The consequences of such a choice are prediction of rate 161 162 dependent (creep) swelling under isotropic unloading for positive (compressive) mean stress 163 (but negative  $\chi_p$ ), and a critical state line in *p*-*q* space (*M*) corresponding only to the reference rate. I.e. interpreted negative "cohesion" (interception of the M line with the q axis) for 164 undrained test ran slower than the reference and a positive interpreted "cohesion" for tests ran 165 166 faster than the reference. The differences between the two alternative options are summarized 167 in Figure 1.

168

# 169 CONCLUSIONS AND RECOMMENDATIONS

170 The classical creep model formulation, widely used for clay, was successfully derived using 171 the framework of hyper-viscoplasticity. The derived flow potential was compared to existing 172 formulations and classical model parameters were identified. The consequences of the choice 173 of free energy function and force potential to include the so-called shift stress are highlighted

| 174 | and compared to the alternative form. Further developments in constitutive modelling of          |                                    |  |  |  |  |
|-----|--|------------------------------------|--|--|--|--|
| 175 | clays using the framework of hyper-viscoplasticity can be done along the same lines, as          |                                    |  |  |  |  |
| 176 | presented here. This means inclusion of e.g. fabric, structure and Lode angle dependency is      |                                    |  |  |  |  |
| 177 | quite straightforward. For modelling of cyclic behavior of clay, it should be noted that         |                                    |  |  |  |  |
| 178 | including a plastic part of the free energy (with some similarities as in alt.2) is essential to |                                    |  |  |  |  |
| 179 | define a kinematic hardening mechanism in a multisurface formulation as like e.g.                |                                    |  |  |  |  |
| 180 | Likitlersuang and Houlsby (2007).  |                                    |  |  |  |  |
| 181 |  |                                    |  |  |  |  |
| 182 | ACKNOWLEDGEMENTS   |                                    |  |  |  |  |
| 183 | This work was supported by the Research council of Norway through its Centers of                 |                                    |  |  |  |  |
| 184 | Excellence funding Scheme, PoreLab, project number 262644.                                       |                                    |  |  |  |  |
| 185 |  |                                    |  |  |  |  |
| 186 | NOTATION   |                                    |  |  |  |  |
| 187 | d  | dissipation function               |  |  |  |  |
| 188 | f  | Helmholtz free energy function     |  |  |  |  |
| 189 | G  | Shear stiffness                    |  |  |  |  |
| 190 | g  | Gibbs free energy function         |  |  |  |  |
| 191 | n  | power number                       |  |  |  |  |
| 192 | OCR  | Over Consolidation Ratio           |  |  |  |  |
| 193 | р  | mean effective stress              |  |  |  |  |
| 194 | $p_0$  | isotropic pre-consolidation stress |  |  |  |  |

| 195 | p <sub>0ref</sub>                  | reference isotropic pre-consolidation stress            |
|-----|------------------------------------|---|
| 196 | p <sub>eq</sub>                    | equivalent effective stress                             |
| 197 | pref                               | reference pressure                                      |
| 198 | q                                  | deviatoric stress                                       |
| 199 | r                                  | reference rate  |
| 200 | w                                  | flow potential function                                 |
| 201 | У                                  | yield surface   |
| 202 | Ζ                                  | force potential function                                |
| 203 | $\mathcal{E}_{\mathcal{V}}$        | volumetric strain                                       |
| 204 | $\mathcal{E}_q$                    | deviatoric strain                                       |
| 205 | $oldsymbol{\mathcal{E}}_{ij}^{vp}$ | viscoplastic strain tensor                              |
| 206 | $\mathcal{E}_{v}^{vp}$             | volumetric viscoplastic strain                          |
| 207 | ${\cal E}_q^{vp}$                  | deviatoric viscoplastic strain                          |
| 208 | $\eta_{K0NC}$                      | stress ratio, $q/p$ , given under 1D normal compression |
| 209 | κ                                  | elastic compressibility parameter (strain based)        |
| 210 | $(\lambda - \kappa)$               | ) plastic compressibility parameter (strain based)      |
| 211 | $\dot{\lambda}_{ref}$              | reference rate for viscoplastic multiplier              |
| 212 | М                                  | critical state line in <i>p</i> - <i>q</i> space        |
| 213 | μ                                  | creep number (strain based)                             |

| 214 | $\sigma_{ij}$  | true stress tensor   |     |  |  |
|-----|--|--|-----|--|--|
| 215 | τ  | reference time   |     |  |  |
| 216 | Xij  | dissipative generalized stress tensor  |     |  |  |
| 217 | χp   | dissipative generalized mean stress  |     |  |  |
| 218 | χq   | dissipative generalized deviatoric stress  |     |  |  |
| 219 | $\chi \overline{ij}$   | generalized stress tensor  |     |  |  |
| 220 | $\chi_p$   | generalized mean stress  |     |  |  |
| 221 | $\chi \overline{q}$  | generalized deviatoric stress  |     |  |  |
| 222 |  |  |     |  |  |
| 223 | APPE   | ENDIX: THE PRINCIPLES OF HYPER-VISCOPLASTICITY [ADAPTED FROM   |     |  |  |
| 224 | THE  | BOOK OF Houlsby and Puzrin (2006)]   |     |  |  |
| 225 | First 1  | law of thermodynamics says that the change (with time) in internal energy, $\dot{U}$ , is equ            | al  |  |  |
| 226 | to the sum of applied heat $\dot{Q}$ and applied work $\dot{W}$ .  |  |     |  |  |
| 227 | <u></u><br><u></u><br><u></u><br><u></u><br><u></u><br><u></u><br><u></u><br><u></u><br><u></u><br><u></u>     | $\dot{Q} + \dot{W}$  | (26 |  |  |
| 228 | For the case of mechanical behavior of materials, it is more natural to speak of specific                      |  |     |  |  |
| 229 | internal energy, $\dot{u}$ (energy per volume and time, kW/m <sup>3</sup> = kPa/s). In this case the change in |  |     |  |  |
| 230 | work,  | , with time, (i.e. the power) is simply the stress times the strain rate:                                |     |  |  |
| 231 | $\dot{W} = 0$  | σ∶έ  | (27 |  |  |
| 232 | Wher   | e $\sigma$ is the Cauchy stress tensor and $\varepsilon$ is the increment in the work conjugative strain |     |  |  |
| 233 | tensor   | r (i.e. the Almansi strain). However, in practice (for small strains) the increment of                   |     |  |  |

(26)

(27)

Almansi strain matches the engineering strain increment, calculated on the updated geometry.The specific change in heat is:

$$236 \qquad \dot{Q} = -\nabla \cdot \mathbf{q} \tag{28}$$

237 Where **q** is the gradient of heat (heat flux). The second law of thermodynamics in terms of 238 specific entropy,  $\dot{s}$ , can be written as:

$$239 \qquad \dot{s} \ge -\nabla \cdot \frac{\mathbf{q}}{\theta} \tag{29}$$

Also called the Clausius-Duhem inequality (entropy increase). Where  $\theta$  is the temperature.

# 241 This can be rewritten as:

242 
$$\dot{s} \ge -\frac{\nabla \cdot \mathbf{q}}{\theta} + \frac{\mathbf{q} \cdot \nabla \theta}{\theta^2}$$
 (30)

243 Or:

244 
$$d = \theta \dot{s} + \nabla \cdot \mathbf{q} \ge \frac{\mathbf{q} \cdot \nabla \theta}{\theta}$$
(31)

245 Where *d* is the dissipation. Since one can assume small temperature gradients in most

246 geotechnical problems, eq. (31) can be rewritten to a more strict condition:

$$247 \qquad d \ge 0 \tag{32}$$

248 The specific internal energy can now be written as:

249 
$$\dot{u} = \boldsymbol{\sigma} : \dot{\boldsymbol{\varepsilon}} - \nabla \cdot \boldsymbol{q} = \boldsymbol{\sigma} : \dot{\boldsymbol{\varepsilon}} + \theta \dot{\boldsymbol{s}} - d$$
 (33)

250 Which by using the assumption of small increments must be equal to:

251 
$$\dot{u} = \frac{\partial u}{\partial \varepsilon} : \dot{\varepsilon} + \frac{\partial u}{\partial s} : \dot{s} + \frac{\partial u}{\partial \varepsilon^{vp}} : \dot{\varepsilon}^{vp}$$
 (34)

Each term is identified as:

$$\boldsymbol{\sigma} = \frac{\partial u}{\partial \boldsymbol{\varepsilon}}$$
253 
$$\boldsymbol{\theta} = \frac{\partial u}{\partial \boldsymbol{s}}$$

$$\boldsymbol{d} = -\frac{\partial u}{\partial \boldsymbol{\varepsilon}^{vp}} : \boldsymbol{\varepsilon}^{vp} = \overline{\boldsymbol{\chi}} : \boldsymbol{\varepsilon}^{vp}$$
(35)

254 Selecting d as a homogenous function of degree n and using the Euler's theorem for

255 homogeneous functions, one can write:

256 
$$d = \frac{1}{n} \cdot \frac{\partial d}{\partial \dot{\boldsymbol{\varepsilon}}^{vp}} : \dot{\boldsymbol{\varepsilon}}^{vp} = \boldsymbol{\chi} : \dot{\boldsymbol{\varepsilon}}^{vp} = \frac{\partial z}{\partial \dot{\boldsymbol{\varepsilon}}^{vp}} : \dot{\boldsymbol{\varepsilon}}^{vp}$$
(36)

- 257 Where *z* is defined as the force potential.
- Eliminating d and  $\dot{u}$  between the equations (33), (34) and (36) results in the following
- 259 requirement:

$$260 \quad (\overline{\chi} - \chi): \dot{\varepsilon}^{vp} = 0 \tag{37}$$

261 The simplest choice is that:

$$262 \quad \overline{\chi} = \chi \tag{38}$$

- 263 Called Ziegler's orthogonality assumption (alternatively, orthogonality between  $(\overline{\chi} \chi)$  and
- 264  $\dot{\epsilon}^{\nu p}$  must be satisfied). The ingredients in hyper-viscoplasticity are hence the free energy
- 265 function, the force potential and any potential hardening rules. The remaining formulations in
- 266 more convenient variables follows through derivations/transformations.

267

#### 268 REFERENCES

Apriadi, D., Likitlersuang, S. & Pipatpongsa, T. 2013. Loading path dependence and non-linear
 stiffness at small strain using rate-dependent multisurface hyperplasticity model. *Computers and Geotechnics*, 49, 100-110.

- Aung, Y., Khabbaz, H. & Fatahi, B. 2019. Mixed hardening hyper-viscoplasticity model for soils
   incorporating non-linear creep rate H-creep model. *International Journal of Plasticity*, 120,
   88-114.
- Bjerrum, L. 1967. Engineering Geology of Norwegian Normally-Consolidated Marine Clays as Related
   to Settlements of Buildings. *Géotechnique*, 17, 83-118.
- Buisman, A. Results of long duration settlement tests. Proceedings 1st International Conference on
   Soil Mechanics and Foundation Engineering, Cambridge, Mass, 1936. 103-107.
- Collins, I. F. & Houlsby, G. T. 1997. Application of thermomechanical principles to the modelling of
   geotechnical materials. *Proceedings of the Royal Society of London. Series A: Mathematical, Physical and Engineering Sciences,* 453, 1975-2001.
- Collins, I. F. & Kelly, P. A. 2002. A thermomechanical analysis of a family of soil models.
   *Géotechnique*, 52, 507-518.
- Darabi, M. K., Abu Al-Rub, R. K. & Omidi, O. 2018. A thermodynamically consistent framework to
   derive local/nonlocal generalized nonassociative plasticity/viscoplasticity theories.
   *International Journal of Plasticity*, 110, 19-37.
- 287Grimstad, G. & Degago, S. A. 2010. A non-associated creep model for structured anisotropic clay (n-288SAC). Numerical Methods in Geotechnical Engineering, 2010. CRC Press, 3-8.
- Grimstad, G., Degago, S. A., Nordal, S. & Karstunen, M. 2008. Modelling creep and rate effects using
   the time resistance concept in a model for anisotropy and destructuration. Nordisk
   Geoteknikermøte nr.15, Proceedings, 2008. Norsk Geoteknisk Forening og Tekna, 195-202.
- Grimstad, G., Degago, S. A., Nordal, S. & Karstunen, M. 2010. Modeling creep and rate effects in
   structured anisotropic soft clays. *Acta Geotechnica*, 5, 69-81.
- Grimstad, G., Karstunen, M., Jostad, H. P., Sivasithamparam, N., Mehli, M., Zwanenburg, C., Evert, D.
  H., Ghoreishian Amiri, S. A., Boumezerane, D., Kadivar, M., Haji Ashrafi, M. A. & Rønningen,
  J. A. 2017. Creep of geomaterials some finding from the EU project CREEP. *European Journal of Environmental and Civil Engineering*, 16.
- Houlsby, G. T. & Puzrin, A. M. 2006. *Principles of hyperplasticity: an approach to plasticity theory based on thermodynamic principles,* Springer Science & Business Media.
- Janbu, N. 1969. The resistance concept applied to deformations of soils. 7th International
   Conference Soil Mechanics Foundation Engineering, 1969 Mexico city. 191–196.
- Leoni, M., Karstunen, M. & Vermeer, P. A. 2008. Anisotropic creep model for soft soils.
   *Géotechnique*, 58, 215-226.
- Likitlersuang, S. & Houlsby, G. T. 2007. Predictions of a continuous hyperplasticity model for Bangkok
   clay. *Geomechanics and Geoengineering*, 2, 147-157.
- 306 Osman, A. S., Birchall, T. J. & Rouainia, M. 2020. A simple model for tertiary creep in geomaterials.
   307 *Geotechnical Research*, 7, 26-39.
- Roscoe, K. H. & Burland, J. B. 1968. On the generalized stress–strain behaviour of wet clay.
   Engineering Plasticity, 1968. Cambridge, 535-609.
- Stolle, D. F. E., Vermeer, P. A. & Bonnier, P. G. 1999. A consolidation model for a creeping clay.
   *Canadian Geotechnical Journal*, 36, 754-759.
- Šuklje, L. 1963. The Equivalent elastic Constants of Saturated Soils Exhibiting Anisotropy and Creep
   Effects. *Géotechnique*, 13, 291-309.
- 314 Ziegler, H. 1983. *An Introduction to Thermomechanics*, North-Holland.
- 315

316

317 Figures:

318

- 319 Figure 1 Graphical representation of the two different presented options, alt. 1 without shift
- 320 stress, alt. 2 with shift stress.

