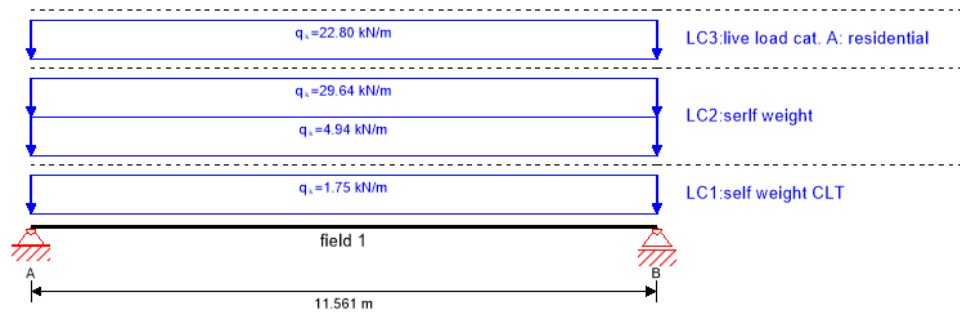


## system



## global utilization ratio

97 %

ULS	97 %	ULS fire	34 %	SLS	86 %	SLS vibration	0 %	support	-1 %
-----	------	----------	------	-----	------	---------------	-----	---------	------

## section: wooden beam 35/100

	section width	section height	area	$I_y$	$I_z$
	[cm]	[cm]	[mm <sup>2</sup> ]	[mm <sup>4</sup> ]	[mm <sup>4</sup> ]
	35	100	350,000	29,166,670,000	3,572,916,000

## section fire: wooden beam 35/100

	section width	section height	area	$I_y$	$I_z$
	[cm]	[cm]	[mm <sup>2</sup> ]	[mm <sup>4</sup> ]	[mm <sup>4</sup> ]
	35	100	350,000	29,166,670,000	3,572,916,000

fire resistance class: R 0	time	0 min				
fire protection layering : no additional fire protection	$k_0$	$d_0$	$d_{char,0,h}$	$d_{ef,h}$	$d_{char,0,v}$	$d_{ef,v}$
	[-]	[mm]	[mm]	[mm]	[mm]	[mm]
	1	7	0.0	0.0	0.0	0.0

## material values

material	$f_{m,k}$	$f_{t,0,k}$	$f_{t,90,k}$	$f_{c,0,k}$	$f_{c,90,k}$	$f_{v,k}$	$f_{r,k \min}$	$E_{0,mean}$	$G_{mean}$	$E_{0.5}$
	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]	[N/mm <sup>2</sup> ]
GL 32h	32.00	25.60	0.50	32.00	2.50	2.50	1.20	14,200.00	350.00	11,800.00

## load

## load case groups

	load case category	Typ	duration	$K_{mod}$	$\gamma_{inf}$	$\gamma_{sup}$	$\psi_0$	$\psi_1$	$\psi_2$
LC1	self weight CLT	G	permanet	0.6	1	1.35	1	1	1
LC2	serlf weight	G	permanet	0.6	1	1.35	1	1	1
LC3	live load cat. A: residential	Q	medium term	0.8	0	1.5	0.7	0.5	0.3

## LC1:self weight CLT

continous load	
field	load at start
	[kN/m]
1	1.75

### LC2:serlf weight

#### continous load

field	load at start
	[kN/m]
1	4.94
1	29.64

### LC3:live load cat. A: residential

#### continous load

field	load at start
	[kN/m]
1	22.80

### ULS combinations

	combination rule
LCO1	$1.12/1.00 * LC1 + 1.12/1.00 * LC2$
LCO2	$1.12/1.00 * LC1 + 1.12/1.00 * LC2 + 1.25/0.00 * LC3$

### ULS combinations fire

	combination rule
LCO3	$1.00/1.00 * LC1 + 1.00/1.00 * LC2$
LCO4	$1.00/1.00 * LC1 + 1.00/1.00 * LC2 + 1.00/0.00 * 0.30 * LC3$

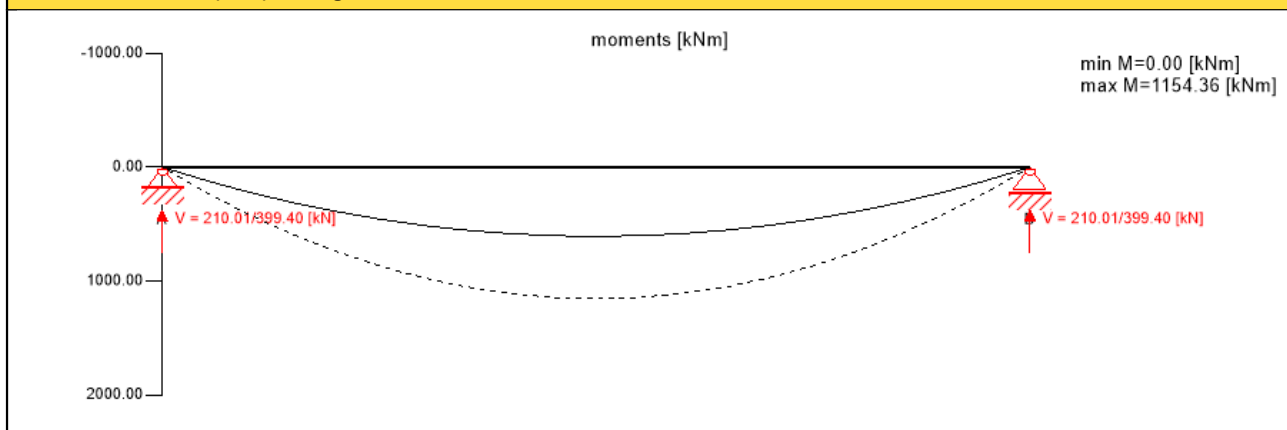
### SLS characteristic combination

	combination rule
LCO5	$1.00/1.00 * LC1 + 1.00/1.00 * LC2$
LCO6	$1.00/1.00 * LC1 + 1.00/1.00 * LC2 + 1.00/0.00 * LC3$

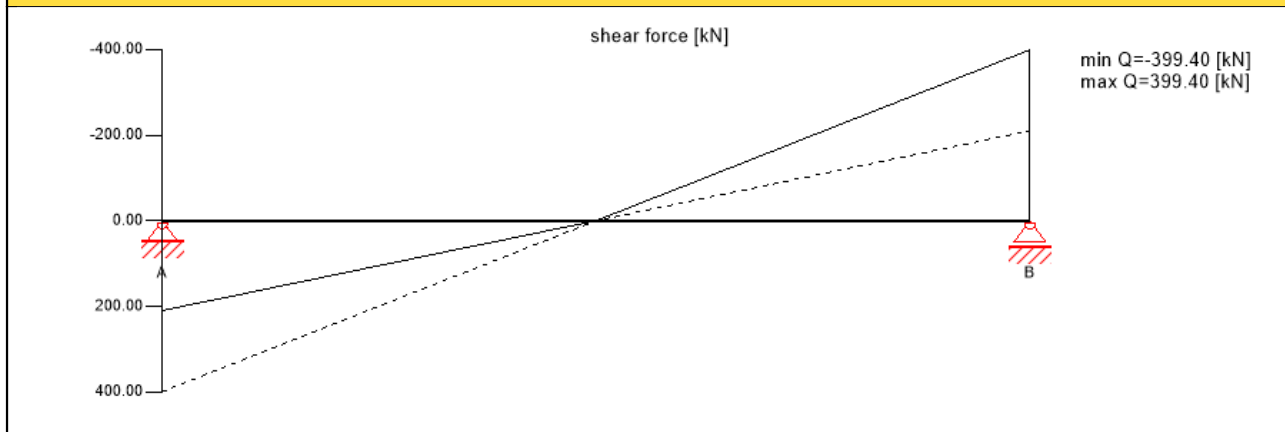
### SLS quasi-permanent combination

	combination rule
LCO7	$1.00/1.00 * LC1 + 1.00/1.00 * LC2$
LCO8	$1.00/1.00 * LC1 + 1.00/1.00 * LC2 + 1.00/0.00 * 0.30 * LC3$

### Ultimate limit state (ULS) - design results



### Ultimate limit state (ULS) - design results



### ULS flexural design

field	dist.	$f_{m,k}$	$\gamma_m$	$k_{mod}$	$k_{sys,z}$	$k_{hm}$	$f_{m,y,d}$	$M_{y,d}$	$\sigma_{m,y,d}$	ratio	
	[m]	[N/mm <sup>2</sup> ]	[-]	[-]	[-]	[-]	[N/mm <sup>2</sup> ]	[kNm]	[N/mm <sup>2</sup> ]		
1	5.78	32.00	1.25	0.80	1.00	1.00	20.48	1154.36	19.79	97 %	LCO2

### ULS shear analysis

field	dist.	$f_{v,k}$	$\gamma_m$	$k_{mod}$	$f_{v,d}$	$V_d$	$T_{v,d}$	ratio	
	[m]	[N/mm <sup>2</sup> ]	[-]	[-]	[N/mm <sup>2</sup> ]	[kN]	[N/mm <sup>2</sup> ]		
1	1.0	2.50	1.25	0.80	1.60	330.30	1.42	88 %	LCO2

### flexural stress analysis

$M_{y,d} =$	1154.36	kNm	$f_{m,k} =$	32.00	N/mm <sup>2</sup>
$N_{t,d} =$	0.00	kN	$\gamma_m =$	1.25	-
			$k_{mod} =$	0.80	-
			$k_{sys,y} =$	1.00	-
			$k_{hm} =$	1.00	-
			$k_l =$	1.00	-
$\sigma_{t,d} =$	0.00	N/mm <sup>2</sup>	$f_{t,d} =$	17.29	N/mm <sup>2</sup>
$\sigma_{m,y,d} =$	19.79	N/mm <sup>2</sup>	$f_{m,y,d} =$	20.48	N/mm <sup>2</sup>

utilization ratio

97 %

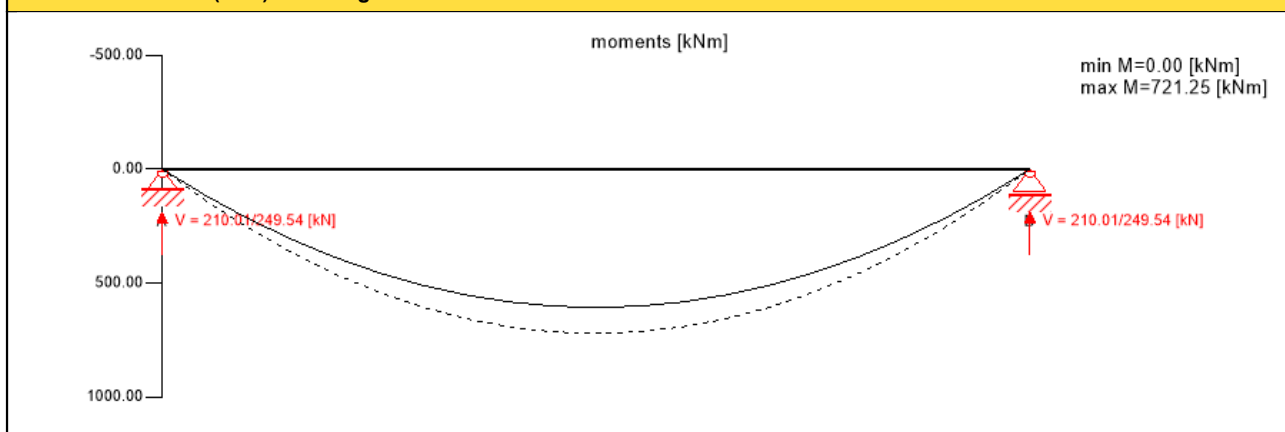
### shear stress analysis

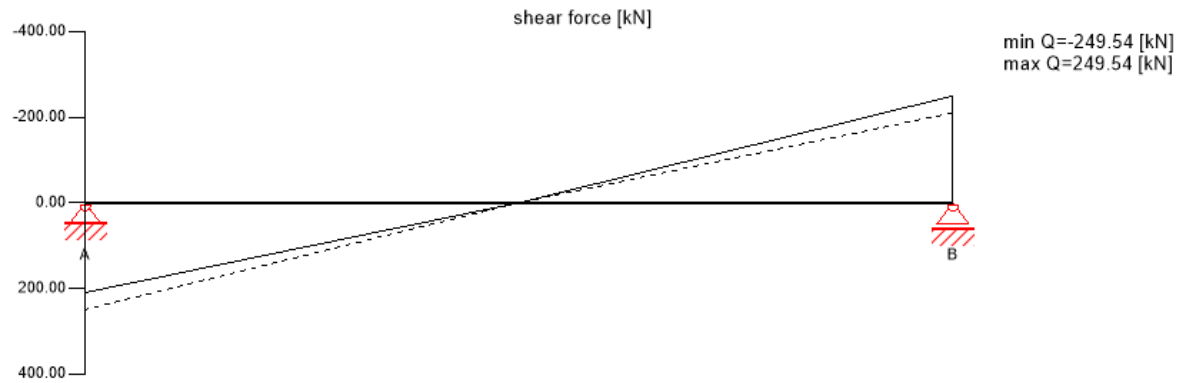
$V_d =$	330.30	kN	$f_{v,k} =$	2.50	N/mm <sup>2</sup>
			$\gamma_m =$	1.25	-
			$k_{mod} =$	0.80	-
$T_{v,d} =$	1.42	N/mm <sup>2</sup>	$f_{v,d} =$	1.60	N/mm <sup>2</sup>

utilization ratio

88 %

### Ultimate limit state (ULS) fire design - results



**Ultimate limit state (ULS) fire design - results****ULS fire flexural design**

field	dist.	$f_{m,k}$	$\gamma_m$	$k_{mod}$	$k_{sys,z}$	$k_{fi}$	$f_{m,y,d}$	$M_{y,d}$	$\sigma_{m,y,d}$	ratio	
	[m]	[N/mm <sup>2</sup> ]	[-]	[-]	[-]	[-]	[N/mm <sup>2</sup> ]	[kNm]	[N/mm <sup>2</sup> ]		
1	5.78	32.00	1.00	1.00	1.00	1.15	36.80	721.25	12.36	34 %	LCO4

**ULS fire shear analysis**

field	dist.	$f_{v,k}$	$\gamma_m$	$k_{mod}$	$k_{fi}$	$f_{v,d}$	$V_d$	$T_{v,d}$	ratio	
	[m]	[N/mm <sup>2</sup> ]	[-]	[-]	[-]	[N/mm <sup>2</sup> ]	[kN]	[N/mm <sup>2</sup> ]		
1	1.0	2.50	1.00	1.00	1.15	2.88	206.37	0.88	31 %	LCO4

**flexural stress analysis fire**

$M_{y,d} =$	721.25	kNm	$f_{m,k} =$	32.00	N/mm <sup>2</sup>
$N_{t,d} =$	0.00	kN	$\gamma_m =$	1.00	-
			$k_{mod} =$	1.00	-
			$k_{sys,y} =$	1.00	-
			$k_{hm} =$	1.00	-
			$k_l =$	1.00	-
			$k_{fi} =$	1.15	-
$\sigma_{t,d} =$	0.00	N/mm <sup>2</sup>	$f_{t,d} =$	31.07	N/mm <sup>2</sup>
$\sigma_{m,y,d} =$	12.36	N/mm <sup>2</sup>	$f_{m,y,d} =$	36.80	N/mm <sup>2</sup>

utilization ratio

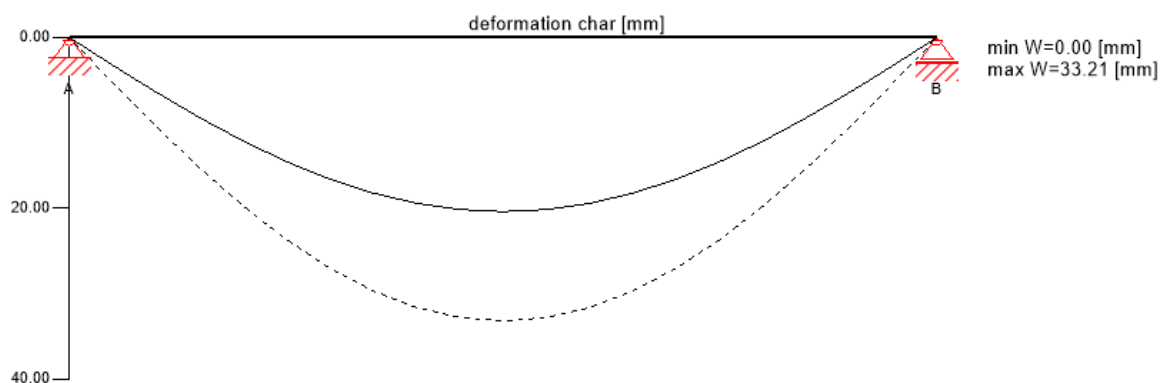
34 %

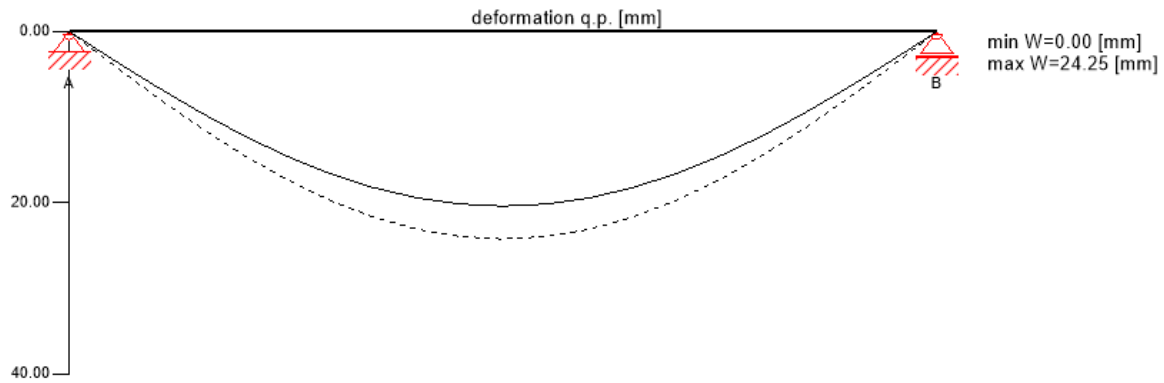
**shear stress analysis fire**

$V_d =$	206.37	kN	$f_{v,k} =$	2.50	N/mm <sup>2</sup>
			$\gamma_m =$	1.00	-
			$k_{mod} =$	1.00	-
			$k_{fi} =$	1.15	-
$T_{v,d} =$	0.88	N/mm <sup>2</sup>	$f_{v,d} =$	2.88	N/mm <sup>2</sup>

utilization ratio

31 %

**Service limit state design (SLS) - design results**

**Service limit state design (SLS) - design results** **$w_{inst} = w[char]$** 

field	limit	$w_{limit}$	$w_{calc.}$	ratio
	[-]	[mm]	[mm]	
1	1/300	38.5	33.2	86 %

 **$w_{fin} = w[char] + w[q.p.]*k_{def}$** 

field	limit	$w_{limit}$	$w_{calc.}$	ratio
	[-]	[mm]	[mm]	
1	1/150	77.1	47.8	62 %

 **$w_{net,fin} = w[q.p.] + w[q.p.]*k_{def}$** 

field	limit	$w_{limit}$	$w_{calc.}$	ratio
	[-]	[mm]	[mm]	
1	1/250	46.2	38.8	84 %

**support reaction**

load case category	$k_{mod}$	$A_v$	$B_v$
		[kN]	
self weight CLT	0.6	10.12	10.12
		10.12	10.12
self weight	0.6	199.89	199.89
		199.89	199.89
live load cat. A: residential	0.8	131.80	131.80
		0.00	0.00

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